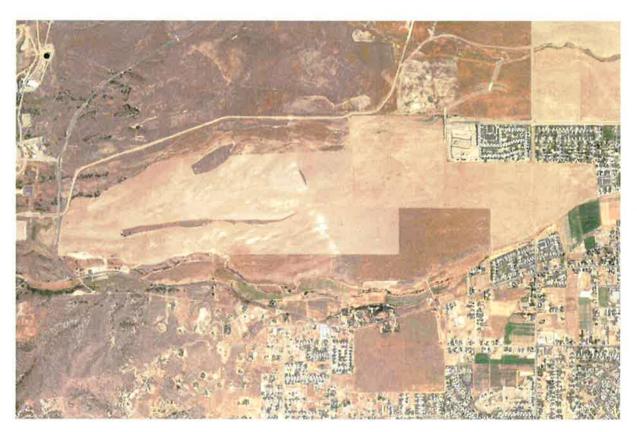
EXHIBIT D

Traffic Impact Study





Traffic Impact Study



Salt Lake County, Utah

December 13, 2019

UT19-1472





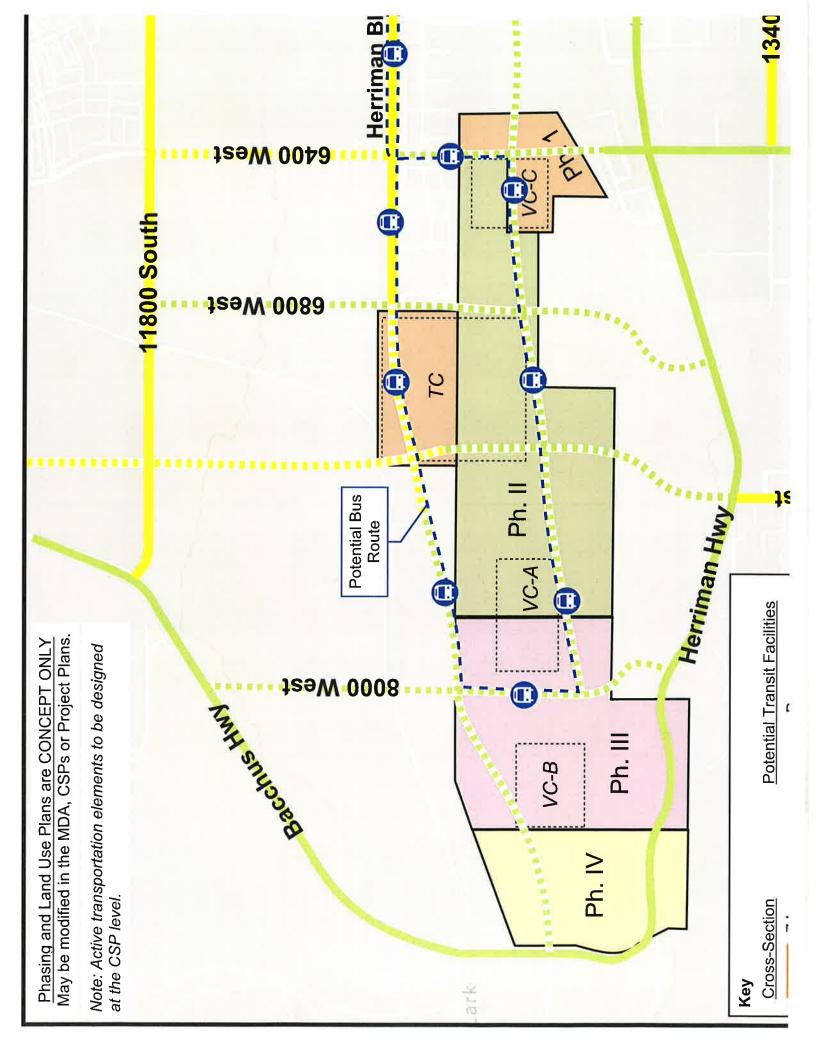
EXECUTIVE SUMMARY

This study addresses the traffic impacts associated with the proposed Olympia Hills development located in Salt Lake County, Utah. The proposed project is located generally between 6400 West and Bacchus Highway on the east and west, and 12600 South and Herriman Highway on the north and south.

Included within the analyses for this study are the traffic operations and recommended mitigation measures for existing (2019) conditions at key intersections and roadways near the project site, and future background conditions starting in 2022 and in five-year increments thereafter. Plus project conditions (conditions after development of the proposed project) were analyzed starting in 2027 and in five-year increments thereafter.

The morning and evening peak hour levels of service (LOS) were computed for each study intersection. LOS A, B, C, and D were considered to be acceptable according to standard practice. When an intersection was anticipated to operate at LOS E or F or when there was excessive queueing, Hales Engineering made recommendations to improve the intersection. In each background and plus project scenario, Hales Engineering was able to mitigate all poor levels of service except for poor LOS at the Mountain View Corridor and Bangerter Highway intersections, which are under UDOT jurisdiction.

A map showing the proposed roadway and transit network is shown in Figure ES-1. A summary of the original and mitigated LOS results for each scenario is shown in Table ES-1. The recommended improvements by scenario are shown in Table ES-2. Additional improvement details are found in Appendix F.



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		BG	BG	BG	РР	BG	ЬР	BG	ЬР	BG	РР
v	Bacchus Hwy / 11800 S			1.1		1.2					
2	7300 W / 11800 S						2.1	2.2	2.3		2.4
3	6400 W / 11800 S					3.1	3.2	3.3	3.4	3.5	3.6
4	6000 W / 11800 S			4.1							
2	Freedom Park Dr / 11800 S				5.1						
9	8000 W / Bacchus Hwy										6.1
∞	6400 W / Herriman Blvd						8.1	8.2	8.3		
10	Anthem Park Blvd / Herriman Blvd				10.1		10.2	10.3		10.4	10.5
11	Main St / Herriman Blvd		11.1	11.2	11.3		11.4			11.5	11.6
12	SB MVC / 12600 S		5.4	12.1	12.2	12.3					
13	NB MVC / 12600 S				13.1	13.2					
14	Bangerter Hwy / 12600 S	14.1	14.2		13.1						
17	8000 W / Herriman Hwy										17.1
18	7300 W / Herriman Hwy						18.1	18.2	20.4		18.3
19	6800 W / Herriman Hwy						19.1				
20	6400 W / Main St			20.1	20.2		20.3		20.4	20.5	20.6
22	6400 W / 13400 S			22.1			22.2		22.3	22.4	
23	5600 W / 13400 S			23.1	23.2		23.3	23.4			23.5
24	5000 W / 13400 S					25.2	24.1				
25	SB MVC / 13400 S	5.6		25.1		25.2					
26	NB MVC / 13400 S	5.6	5.7	26.1		5.18					
27	8000 W / Herriman Blvd										27.1
28	7300 W / Herriman Blvd						28.1	28.2	28.3	28.4	28.5
29	6800 W / Herriman Blvd				-		29.1				
(1)	11800 South					5.1					
1	Herriman Boulevard					5.2					5.3
ax.	12600 South		5.4								
100	Herriman Highway					5.5					
	12100 Cauth	כע	67				٥٥	٥٥			

#	Description
1.1	Signal, Turn Pockets on All Approaches, Dual SB L
1.2	NB RT Lane
2.1	Signal, Turn Pockets on All Approaches
2.2	Signal, Turn Pockets on All Approaches
2.3	PM/PT LT Phasing on All Approaches
2.4	NB & WB Dual Left Turn Lanes
3.1	Signal, Turn Pockets on All Approaches, EB & WB
3.2	EB & WB RT Lanes, NB PM/PT LT Phasing
3.3	EB Dual LT Lanes, NB & SB PM/PT LT Phasing
3.4	PM/PT LT Phasing on All Approaches, NB Dual LT
3.5	EB & WB RT Lanes
3.6	Add SB Thru Lane, Convert NB RT Lane to Shared
4.1	WB PM/PT LT Phasing, Extend WB LT Storage
5.1	WB Dual LT Lanes
6.1	Signal, Turn Pockets on All Approaches
8.1	Signal, Turn Pockets on All Approaches, EB & WB
8.2	Signal, Turn Pockets on All Approaches
8.3	PM/PT LT Phasing on All Approaches
10.1	WB RT Pocket, Extend LT Lanes on All Approaches
10.2	Convert NB RT to Shared T/RT
10.3	EB Dual LT Lanes
10.4	EB & WB RT Lanes
10.5	NB & WB Dual LT Lanes, Extend NB LT Lane, Conv Shared T/RT Lanes
11.1	NB Channelized RT Lane
11.2	SB Dual LT Lanes
11.3	WB Dual LT Lanes
11.4	Additional NB Thru lane, Convert EB RT to Shared
11.5	2 NB Thru Lanes, WB Dual LT Lanes, Lengthen SB
11.6	EB RT Lane, Add WB Thru Lane
12.1	SB & WB Dual LT Lanes
12.2	Freeway Grade- Separated Interchange
12.3	Channelized EB & WB RT Lanes
13.1	Freeway Grade- Separated Interchange, Addition: Channelized EB/WB RT Lanes
13.2	Channelized EB & WB RT Lanes, Convert NB Thru
14.1	SPUI
14.2	Additional LT Lane on NB Off Ramp



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I. INTRODUCTION

A. Purpose

This study addresses the traffic impacts associated with the proposed Olympia Hills development located in Salt Lake County, Utah. The proposed project is located generally between 6400 West and Bacchus Highway on the east and west, and 12600 South and Herriman Highway on the north and south. Figure 1 shows a vicinity map of the proposed development.

Included within the analyses for this study are the traffic operations and recommended mitigation measures for existing (2019) conditions at key intersections and roadways near the project site, and future background conditions starting in 2022 and in five-year increments thereafter. Plus project conditions (conditions after development of the proposed project) were analyzed starting in 2027 and in five-year increments thereafter.



Figure 1: Vicinity map showing the project location in Salt Lake County, Utah



B. Scope

The study area was defined based on conversations with Salt Lake County staff. This study was scoped to evaluate the traffic operational performance impacts of the project on the following intersections:

- Bacchus Highway / 11800 South
- 6000 West / 11800 South
- Freedom Park Drive / 11800 South
- Bingham Canyon Mine / Bacchus Highway
- 6000 West / Herriman Boulevard
- Anthem Park Boulevard / Herriman Boulevard
- Main Street / Herriman Boulevard / 12600 South
- Mountain View Corridor (SR-85) / 12600 South
- Bangerter Highway (SR-154) / 12600 South
- Silver Sky Drive / 6000 West
- Butterfield Canyon Road / Herriman Highway / Bacchus Highway
- 7300 West / Herriman Highway
- 6400 West / Main Street
- 5600 West / Main Street
- 6400 West / 13400 South
- 5600 West / 13400 South
- 5000 West / 13400 South
- Mountain View Corridor (SR-85) / 13400 South

C. Analysis Methodology

Level of service (LOS) is a term that describes the operating performance of an intersection or roadway. LOS is measured quantitatively and reported on a scale from A to F, with A representing the best performance and F the worst. Table 1 provides a brief description of each LOS letter designation and an accompanying average delay per vehicle for both signalized and unsignalized intersections. Figure 2 provides a visual representation of each LOS letter designation.

The *Highway Capacity Manual* (HCM), 6th Edition, 2016 methodology was used in this study to remain consistent with "state-of-the-practice" professional standards. This methodology has different quantitative evaluations for signalized and unsignalized intersections. For signalized and all-way stop intersections, the LOS is provided for the overall intersection (weighted average of all approach delays). For all other unsignalized intersections, LOS is reported based on the worst approach.



Using Synchro/SimTraffic software, which follow the HCM methodology, the peak hour LOS was computed for each study intersection. Multiple runs of SimTraffic were used to provide a statistical evaluation of the interaction between the intersections. The detailed LOS reports are provided in Appendix D. Hales Engineering also calculated the 95th percentile queue lengths for each of the study intersections using SimTraffic. The detailed queue length reports are provided in Appendix E.



Table 1: Level of Service Description

Level of Service	Description of Traffic Conditions	Average Delay (seconds/vehicle)				
U 7 8 7 F	Signalized Intersections	Overall Intersection				
А	Extremely favorable progression and a very low level of control delay. Individual users are virtually unaffected by others in the traffic stream.	0 ≤ 10.0				
В	Good progression and a low level of control delay. The presence of other users in the traffic stream becomes noticeable.	> 10.0 and ≤ 20.0				
С	Fair progression and a moderate level of control delay. The operation of individual users becomes somewhat affected by interactions with others in the traffic stream.	>20.0 and ≤ 35.0				
D	Marginal progression with relatively elevated levels of control delay. Operating conditions are noticeably more constrained.	> 35.0 and ≤ 55.0				
Е	Poor progression with unacceptably elevated levels of control delay. Operating conditions are at or near capacity.					
F	Unacceptable progression with forced or breakdown operating conditions.	> 80.0				
	Unsignalized Intersections	Worst Approach				
Α	Free Flow / Insignificant Delay	0 ≤ 10.0				
В	Stable Operations / Minimum Delays	>10.0 and ≤ 15.0				
С	Stable Operations / Acceptable Delays	>15.0 and ≤ 25.0				
D	Approaching Unstable Flows / Tolerable Delays	>25.0 and ≤ 35.0				
E	Unstable Operations / Significant Delays Can Occur	>35.0 and ≤ 50.0				
F	Forced Flows / Unpredictable Flows / Excessive Delays Occur	> 50.0				

Source: Hales Engineering Descriptions, based on the *Highway Capacity Manual* (HCM), 6th Edition, 2016 Methodology (Transportation Research Board)



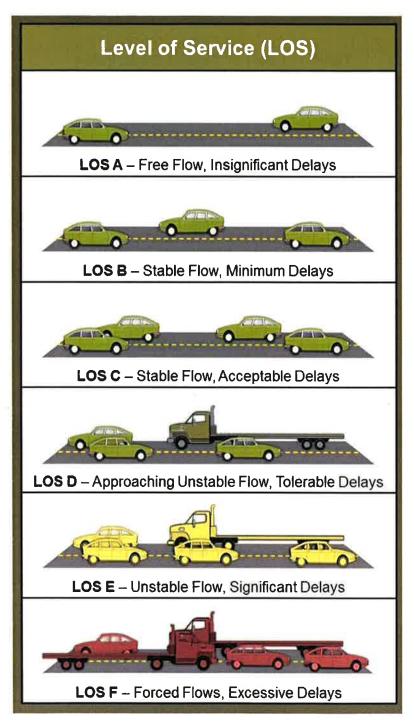


Figure 2: Visual representation of the LOS letter designations



II. EXISTING (2019) BACKGROUND CONDITIONS

A. Purpose

The purpose of the background analysis is to study the intersections and roadways during the peak travel periods of the day with background traffic and geometric conditions. Through this analysis, background traffic operational deficiencies can be identified, and potential mitigation measures recommended. This analysis provides a baseline condition that may be compared to the build conditions to identify the impacts of the development.

B. Roadway System

The primary roadways that will provide access to the project site are described below:

<u>11800 South</u> – is a city-maintained (South Jordan/Herriman) roadway that runs east/west between Bacchus Highway and Mountain View Corridor (SR-85). 11800 South currently consists of a five-lane cross section east of 6000 West, and a two-lane cross section west of 6000 West. The posted speed limit is 35 mph in the study area.

According to Wasatch Choice 2050, the regional transportation plan (RTP) published in 2019 by the Wasatch Front Regional Council (WFRC), 11800 South between 6000 West and Bacchus Highway is planned to be widened to five lanes. This is planned as a Phase 1 (2019-2030) project.

<u>Herriman Boulevard</u> – is a city-maintained (Herriman) roadway that currently extends west from Mountain View Corridor (SR-85) at 12600 South and currently terminates at approximately 6800 West. Herriman Boulevard consists of a five-lane cross section east of 6000 West, and a three-lane cross section west of 6000 West. The posted speed limit is 40 mph in the study area.

According to the WFRC RTP, Herriman Boulevard is planned to be extended west to connect to Bacchus Highway. This is planned as a Phase 1 (2019-2030) project.

Herriman Highway/Main Street – is a county/city-maintained (Salt Lake County/Herriman) roadway that runs east/west between Bacchus Highway and Herriman Boulevard (12600 South near Mountain View Corridor (SR-85). The roadway consists of a two-lane cross section between Bacchus Highway and approximately 6200 West, a three-lane cross section between 6200 West and 5600 West, and a five-lane cross section between 5600 West and Herriman Boulevard (12600 South). The posted speed limit is 35 mph east of 5600 West and 30 mph west of 5600 West.



The segment of Main street between Herriman Boulevard (12600 South) and Anthem Park Boulevard is planned to be completed by the end of 2019.

<u>Bacchus Highway</u> – is a north/south route that spans the entire west bench of the Salt Lake Valley, connecting to SR-201 on the north and Herriman Highway on the south. Bacchus Highway is a county-maintained (Salt Lake County) roadway within the study area. The roadway consists of a two-lane cross section and the posted speed limit is 50 mph within the study area.

Although no formal plan has been adopted, there are talks at the County level about realigning Bacchus Highway south of Old Bingham Highway through the study area. For this study it was assumed that the New Bacchus Highway would deviate from the current alignment near the Trans Jordan Landfill, follow a generally north/south route, and connect to Herriman Highway at 7300 West. It was also assumed that the existing Bacchus Highway would remain and will be referred to in this study as the Old Bacchus Highway once the new alignment is completed.

Other roadways included in this study are described below:

Mountain View Corridor (SR-85) – is a state-maintained roadway (classified by UDOT access management standards as a "Freeway – One-Way Frontage Road" facility, or access category 10 roadway). Mountain View Corridor (SR-85) has two travel lanes in each direction with left- and right-turn lanes at intersections. The north- and southbound lanes are currently separated by a wide median. In the future, a freeway facility will be constructed in this median resulting in a freeway/frontage road system. As identified and controlled by UDOT, a "Freeway – One-Way Frontage Road" access classification identifies minimum signalized intersection spacing of one-quarter mile (1,320 feet), minimum unsignalized street spacing of 660 feet. The posted speed limit on Mountain View Corridor (SR-85) is 55 mph in the study area.

Bangerter Highway (SR-154) – is a state-maintained roadway (classified by UDOT access management standards as a "Freeway/Interstate System" facility, or access category 1 roadway). Bangerter Highway (SR-154) has three travel lanes in each direction with left- and right-turn lanes at intersections and the posted speed limit is 60 mph in the study area. North- and southbound traffic are separated by a raised center median and access is currently limited to signalized intersections or interchanges at major cross streets. According to the WFRC RTP, five at-grade intersections on Bangerter Highway (SR-154) are planned to be converted to grade-separated interchanges as Phase 1 (2019-2030) projects, including at 12600 South.

C. Traffic Volumes

Weekday morning (7:00 to 9:00 a.m.) and evening (4:00 to 6:00 p.m.) peak period traffic counts were performed at the following intersections:

• Bacchus Highway / 11800 South

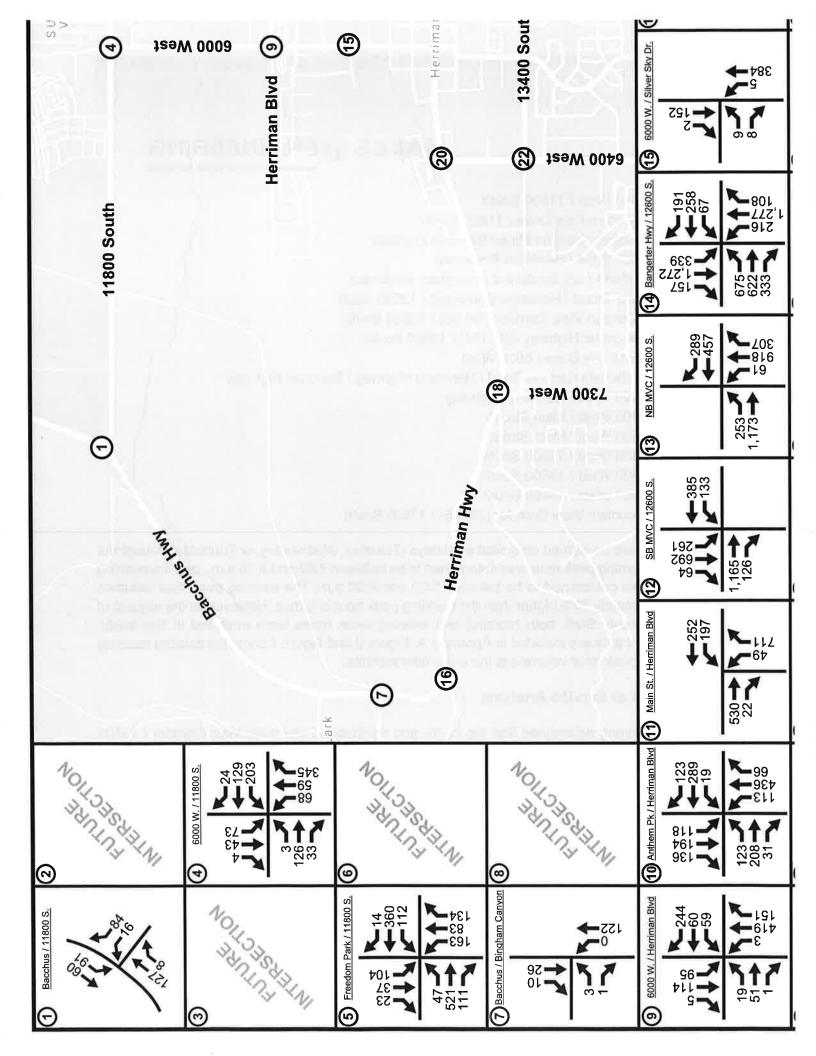


- 6000 West / 11800 South
- Freedom Park Drive / 11800 South
- Bingham Canyon Mine / Bacchus Highway
- 6000 West / Herriman Boulevard
- Anthem Park Boulevard / Herriman Boulevard
- Main Street / Herriman Boulevard / 12600 South
- Mountain View Corridor (SR-85) / 12600 South
- Bangerter Highway (SR-154) / 12600 South
- Silver Sky Drive / 6000 West
- Butterfield Canyon Road / Herriman Highway / Bacchus Highway
- 7300 West / Herriman Highway
- 6400 West / Main Street
- 5600 West / Main Street
- 6400 West / 13400 South
- 5600 West / 13400 South
- 5000 West / 13400 South
- Mountain View Corridor (SR-85) / 13400 South

The counts were performed on typical weekdays (Tuesday, Wednesday, or Thursday) throughout 2019. The morning peak hour was determined to be between 7:00 and 8:00 a.m., and the evening peak hour was determined to be between 5:00 and 6:00 p.m. The evening peak hour volumes were approximately 30% higher than the morning peak hour volumes. However, at the request of Salt Lake County Staff, both morning and evening peak hours were analyzed in this study. Detailed count data are included in Appendix A. Figure 3 and Figure 4 show the existing morning and evening peak hour volumes at the study intersections.

D. Level of Service Analysis

Hales Engineering determined that the north- and southbound Mountain View Corridor / 13400 South intersections are currently operating at LOS E during the morning peak hour as shown in Table 2, and the Bangerter Highway / 12600 South intersection is operating at LOS F during the evening peak hour as shown in Table 3. All other study intersections are currently operating at acceptable levels of service. These results serve as a baseline condition for the impact analysis of the proposed development during existing (2019) conditions.



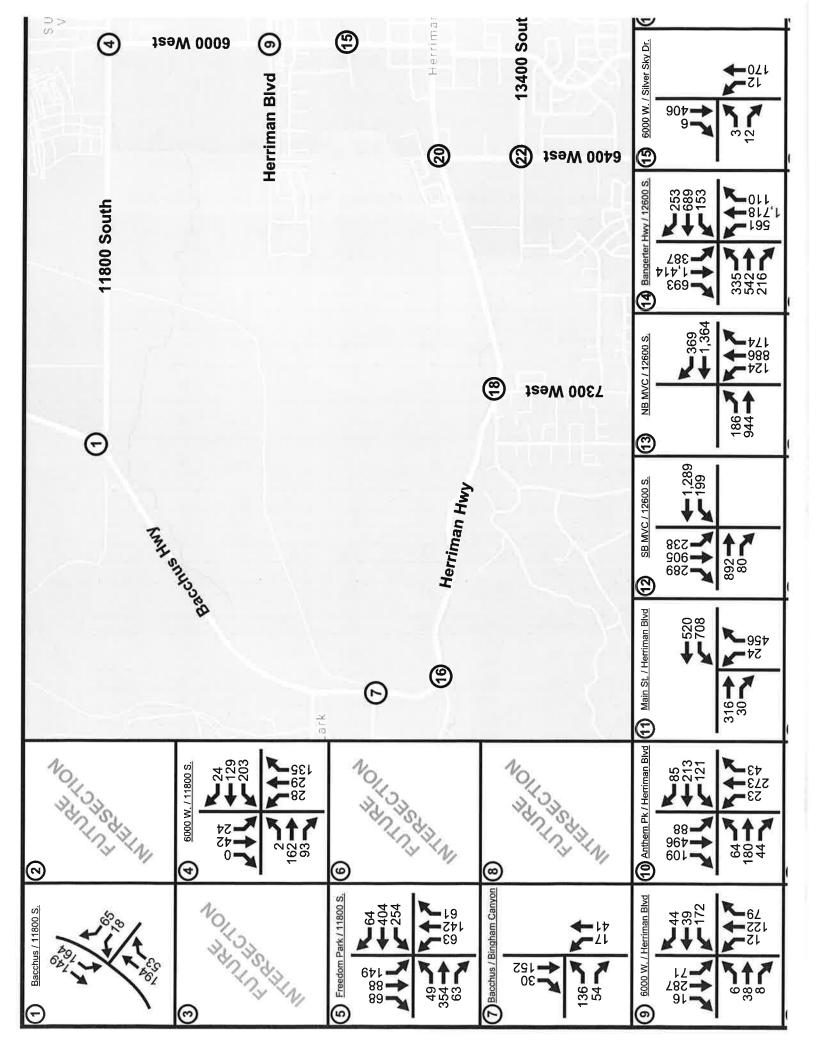




Table 2: Existing (2019) Background Morning Peak Hour Level of Service

Intersection	- N. B.	Wor	st Approach	- 5	Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS1	Aver. Delay (Sec/Veh) ²	LOS²	LOS (Delay)
Bacchus Highway / 11800 South	WB Stop	WB	2.6	Α		•	(E
6000 West / 11800 South	Signal	ē	?≆:	8	11.3	В	925
Freedom Park Drive / 11800 South	Signal	-	:=2	:::::::::::::::::::::::::::::::::::::::	11.3	В	
Bingham Canyon Mine / Bacchus Highway	EB Stop	EB	2.6	A	%	196	*
6000 West / Herriman Boulevard	Signal	-	-	2.6	10.6	В	(**)
Anthem Park Boulevard / Herriman Boulevard	Signal	•	%	%≥	15.4	В	Øi
Main Street / Herriman Boulevard / 12600 South	Signal	-			11.2	В	æ
SB MVC / 12600 South	Signal		() = :	18	22.2	С	
NB MVC / 12600 South	Signal	3	7 E	74	24.2	С	*
Bangerter Highway / 12600 South	Signal		:(=:	(*	47.0	D	
Silver Sky Drive / 6000 West	EB Stop	EB	4.6	Α	(*	•	
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	ЕВ	2.1	A	領	3 #	F 6
7300 West / Herriman Highway	NB Stop	NB	5.6	A	*	(*)	16
6400 West / Main Street	NB/SB Stop	NB	13.0	В	3 5 5	1975	
5600 West / Main Street	Signal	(m)	-	-	20.0	В	
6400 West / 13400 South	Signal	3.	B	5	11.3	В	
5600 West / 13400 South	Signal	4 5	<u> </u>	•	23.2	С	
5000 West / 13400 South	Signal	•			34.4	С	81
SB MVC / 13400 South	Signal	56)	*	•	59.0	E	D (43.5)
NB MVC / 13400 South	Signal	59.0	-	2	56.7	E	C (30.1)

1. This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.

Source: Hales Engineering, October 2019

Table 3: Existing (2019) Background Evening Peak Hour Level of Service

Intersection		Wor	st Approach	V.	Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	WB Stop	NB	5.0	A	**	5 2 3	¥
6000 West / 11800 South	Signal		D#1		11.4	В	
Freedom Park Drive / 11800 South	Signal		1	16	13.5	В	78
Bingham Canyon Mine /	EB Stop	EB	5.3	А			
Bacchus Highway	EB StOP	EB	J.3	А	87	878	
6000 West /	Signal				10.0	А	
Herriman Boulevard	Signal	^	95		10.0	A	
Anthem Park Boulevard / Herriman	Signal				11.5	В	
Boulevard	Signal		N#1		11.5	В	
Main Street / Herriman Boulevard / 12600 South	Signal	*	. 5.	•	7.7	A	/B
SB MVC / 12600 South	Signal	5	::::	1,51	22.8	С	•
NB MVC / 12600 South	Signal	*	8.	•	29.5	С	22
Bangerter Highway / 12600 South	Signal		15 5 1	UBS	83.3	F	C (34.7)
Silver Sky Drive / 6000 West	EB Stop	EB	4.7	A	? = 2	: <u>:</u> ::	:=
Butterfield Canyon Road /							
Herriman Highway /	EB Stop	EB	3.1	A	~	•	-
Bacchus Highway							
7300 West / Herriman Highway	NB Stop	NB	6.1	Α	-	•	/S
6400 West / Main Street	NB/SB	NB	17.1	С		-	
0400 West / Wall Street	Stop	IND	17.1				
5600 West / Main Street	Signal		/c==	(•€)	28.0	С	296
6400 West / 13400 South	Signal	1 2	92	120	15.9	В	12
5600 West / 13400 South	Signal	*		(⊛)	52.0	D	
5000 West / 13400 South	Signal	¥	921	**	20.3	C	12
SB MVC / 13400 South	Signal	•	· ·	£5	41.2	D	() -5
NB MVC / 13400 South	Signal	š		-	48.4	D	

E. **Queuing Analysis**

Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

¹ This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.

Source: Hales Engineering, October 2019



- Mountain View Corridor / 12600 South
 - Westbound Approach 410 feet (a.m. peak)
- Bangerter Highway / 12600 South
 - Significant queueing (approximately 610 feet) on the eastbound approach during the morning peak hour.
 - Significant queueing (>1,000 feet) on the south- and westbound approaches during the evening peak hour.
- 5600 West / Main Street
 - Northbound Approach 365 feet (a.m. peak)
 - Southbound Approach 730 feet (p.m. peak)
- 6400 West / 13400 South
 - Southbound Approach 335 feet (p.m. peak)
- 5600 West / 13400 South
 - Northbound Approach 365 feet (a.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
- 5000 West / 13400 South
 - Eastbound Approach 520 feet (a.m. peak)
 - Westbound Approach 340 feet (p.m. peak)
- Mountain View Corridor / 13400 South
 - Northbound Approach >1,000 feet (a.m. peak)
 - Southbound Approach 590 feet (p.m. peak)
 - Eastbound Approach 525 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.

F. Mitigation Measures

Additional capacity is needed at the Bangerter Highway / 12600 South intersection and the Mountain View Corridor / 13400 South intersection.

As discussed previously, the Bangerter Highway / 12600 South intersection is slated to become a grade-separated interchange before 2030 (Phase 1 Project). According to the State Environmental Study (SES) completed in 2018 by UDOT for this project, construction is anticipated to begin in 2020.

According to the WFRC RTP, Mountain View Corridor south of 13400 South is planned to be widened to three lanes in each direction prior to 2030. (No information could be found regarding a specific construction timeline.



Hales Engineering analyzed a mitigated scenario assuming that both of these improvements had been implemented. By converting the Bangerter Highway / 12600 South intersection to a grade-separated single point urban interchange (SPUI), the intersection is anticipated to operate at an acceptable level of service in both the morning and evening peak hours.

Adding additional lanes to Mountain View Corridor south of 13400 South is not anticipated to improve the level of service at the Mountain View Corridor / 13400 South intersection. It is recommended that an additional eastbound lane be added to 13400 South through the Mountain View Corridor intersection to match the number of eastbound lanes on 13400 South on the east side of Mountain View Corridor. This would provide the needed capacity to accommodate the eastbound demand during the morning peak hour. With this improvement it is anticipated that the Mountain View Corridor / 13400 South intersection will operate at an acceptable level of service during the morning and evening peak hours.

No additional mitigation measures are recommended.



III. FUTURE (2022) BACKGROUND CONDITIONS

A. Purpose

The purpose of the future (2022) background analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions. Through this analysis, future background traffic operational deficiencies can be identified, and potential mitigation measures recommended.

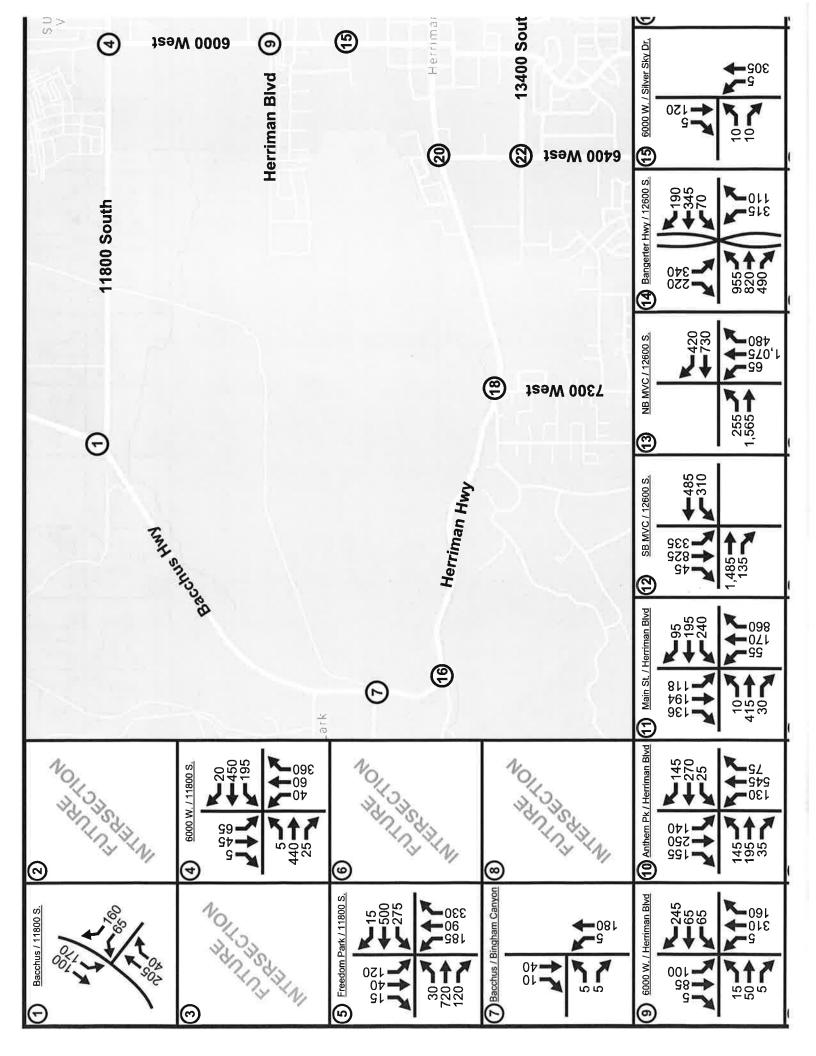
B. Roadway Network

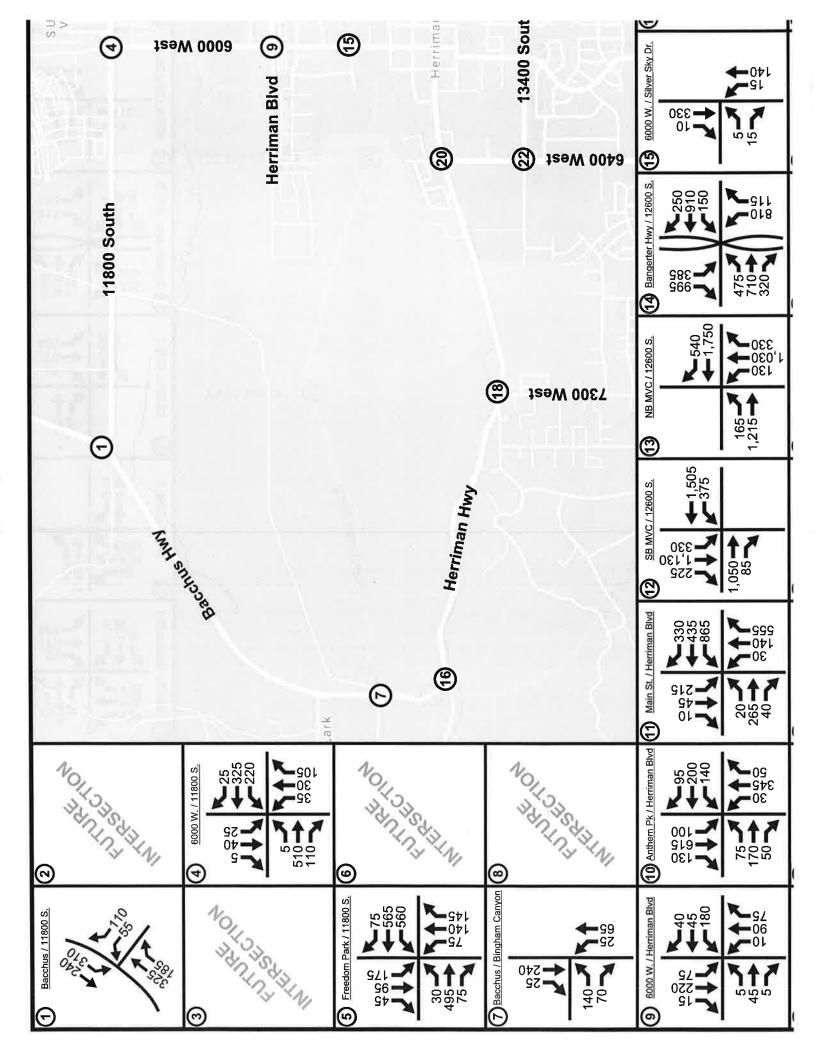
The segment of Herriman Main Street between Herriman Boulevard (12600 South) and Anthem Park Boulevard is currently under construction and is planned to be completed by the end of 2019. It was assumed that this project was completed prior to 2022. It was also assumed that the previously recommended improvements (grade separated interchange at the Bangerter Highway / 12600 South intersection and additional east/west lanes on 13400 South at Mountain View Corridor) had been implemented prior to 2022.

According to the WFRC Regional Transportation Plan, there are several improvement projects in the study area that are planned as Phase 1 (2019-2030) projects. However, none of these improvements were assumed to be completed prior to 2022.

C. Traffic Volumes

Hales Engineering obtained future (2022) forecasted volumes from a modified version of the WFRC / Mountainland Association of Governments (MAG) travel demand model (TDM). This version of the WFRC/MAG TDM was tailored specifically for this project by Horrocks Engineers (and reviewed by Salt Lake County) to forecast future average weekday daily traffic (AWDT) volumes within the study area. Peak period turning movement counts were estimated using National Cooperative Highway Research Program (NCHRP) 255 methodologies which utilize existing peak period turn volumes and future AWDT volumes to project the future turn volumes at the major intersections. Future (2022) morning and evening peak hour turning movement volumes are shown in Figure 5 and Figure 6.







D. Level of Service Analysis

Hales Engineering determined that the Main Street / Herriman Boulevard / 12600 South and southbound Mountain View Corridor / 12600 South intersections are anticipated to operate at LOS E during the morning peak hour in future (2022) background conditions, as shown in Table 4. The southbound Mountain View Corridor / 12600 South intersection is also anticipated to operate at LOS E during the evening peak hour, along with the northbound Mountain View Corridor / 13400 South intersection, as shown in Table 5. These results serve as a baseline condition for the impact analysis of the proposed development for future (2022) conditions.

E. Queuing Analysis

Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Freedom Park Drive / 11800 South
 - Westbound Approach >500 feet (p.m. peak)
- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach 395 feet (a.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Northbound Approach 815 feet (a.m. peak)
 - Southbound Approach 445 feet (a.m. peak)
 - Westbound Approach 555 feet (p.m. peak)
- Mountain View Corridor / 12600 South
 - Northbound Approach 500 feet (a.m. peak)
 - Southbound Approach 480 feet (p.m. peak)
 - Eastbound Approach 665 feet (a.m. peak), 710 feet (p.m. peak)
 - Westbound Approach 695 feet (p.m. peak)
- Bangerter Highway / 12600 South
 - Northbound Offramp >1,000 feet (p.m. peak)
- 6400 West / 13400 South
 - Southbound Approach >1,000 feet (p.m. peak)
- 5600 West / 13400 South
 - Northbound Approach 440 feet (p.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
 - Eastbound Approach 390 feet (p.m. peak)
 - Westbound Approach 575 feet (p.m. peak)
- 5000 West / 13400 South
 - Eastbound Approach 400 feet (a.m. peak)
- Mountain View Corridor / 13400 South
 - Northbound Approach >1,000 feet (a.m. peak), 580 feet (p.m. peak)



- o Southbound Approach 965 feet (p.m. peak)
- Eastbound Approach 465 feet (a.m. peak)
- o Westbound Approach 810 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.



Table 4: Future (2022) Background Morning Peak Hour Level of Service

Intersection		Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS¹	Aver. Delay (Sec/Veh) ²	LOS²	LOS (Delay)
Bacchus Highway / 11800 South	WB Stop	WB	5.7	Α	3(4)	::e:	700
6000 West / 11800 South	Signal		18	1.00	14.2	В	.
Freedom Park Drive / 11800 South	Signal	-	::#:	220	16.0	В	1865
Bingham Canyon Mine / Bacchus Highway	EB Stop	EB	2.8	A	3	٠	<u>16</u>
6000 West / Herriman Boulevard	Signal	3	Ŀĕ	•	9.3	Α	1
Anthem Park Boulevard / Herriman Boulevard	Signal	, Š	ı ğ	Ŗ	18.4	В	76
Main Street / Herriman Boulevard / 12600 South	Signal	ş	Tab	No.	57.5	E	C (26.1)
SB MVC / 12600 South	Signal			*	60.7	Ε	D (48.0)
NB MVC / 12600 South	Signal	•	1.00	5 .	31.4	С	6 9 9
Bangerter Highway / 12600 South	Signal	<u>.</u>	- 1	79	28.1	С	166
Silver Sky Drive / 6000 West	EB Stop	EB	4.1	Α	E=:	39 %	79
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	EB	4.4	Α		*	SE
7300 West / Herriman Highway	NB Stop	NB	6.4	Α		3 . .	(#)
6400 West / Main Street	NB/SB Stop	NB	20.4	С	·	84	·*
5600 West / Main Street	Signal	<u></u>	vē.		15.4	В	(<u>*</u>
6400 West / 13400 South	Signal	¥	041	16	13.4	В	(*C
5600 West / 13400 South	Signal		J.		26.8	С	ě
5000 West / 13400 South	Signal			100	24.8	С	366
SB MVC / 13400 South	Signal		- ē		37.1	D	
NB MVC / 13400 South	Signal	-		- 6	41.1	D	2.80

^{1.} This represents the worst approach LOS and defau (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections

^{2.} This represents the overall intersection LOS and dalay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections

^{3.} SB = Southbound approach, etc

Source: Hales Engineering, October 2019



Table 5: Future (2022) Background Evening Peak Hour Level of Service

Intersection	Worst Approach			Overall Intersection		Mitigated	
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh)	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	WB Stop	WB	19.6	С	=======================================	140	lis
6000 West / 11800 South	Signal		3=3	::€:	14.4	В	
Freedom Park Drive / 11800 South	Signal		<u>@</u>	·	27.4	C	
Bingham Canyon Mine / Bacchus Highway	EB Stop	ЕВ	5.8	Α	:20	•	N=
6000 West / Herriman Boulevard	Signal	Ħ	350	NE	9.2	A	::€:
Anthem Park Boulevard / Herriman Boulevard	Signal	-	9₩9	(€)	14.1	В	:(€
Main Street / Herriman Boulevard / 12600 South	Signal	5	:#:	6 	35.3	D	:=
SB MVC / 12600 South	Signal		S.E.	S	65.8	Е	C (33.9)
NB MVC / 12600 South	Signal	-	24	(%)	34.4	С	Tier
Bangerter Highway / 12600 South	Signal	•	5 .	1572	72.9	E	D (49.0)
Silver Sky Drive / 6000 West	EB Stop	EB	4.4	Α	540	•	i 🙀
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	ЕВ	4.8	A	6 2 3	741	12
7300 West / Herriman Highway	NB Stop	NB	7.1	A	(2)	148	- 6
6400 West / Main Street	NB/SB Stop	NB	17.6	С	(e)	·	-
5600 West / Main Street	Signal		(*	599	19.0	В	
6400 West / 13400 South	Signal		V.	/ -	46.4	D	-
5600 West / 13400 South	Signal			(€	53.0	D	-
5000 West / 13400 South	Signal	ē	7-	18	19.1	В	E
SB MVC / 13400 South	Signal	*	(⊕		39.4	D	
NB MVC / 13400 South	Signal	9.7	19		58.3	Е	D (52.0)

This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.
 This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.
 SB = Southbound approach, etc.

Source: Hales Engineering, October 2019

F. **Mitigation Measures**

The poor level of service during the morning peak hour at the Main Street / Herriman Boulevard / 12600 South intersection can be attributed to high number of right-turning vehicles on the



northbound approach, as well as left-turning vehicles on the southbound approach. It is recommended that a channelized right-turn lane be considered for the northbound right-turn movement.

The poor levels of service during the morning and evening peak hours at the southbound Mountain View Corridor / 12600 South intersection can be attributed to the need for additional eastbound capacity at the intersection. According to the WFRC RTP, an additional lane in each direction is planned to be added to 12600 South between Mountain View Corridor and Bangerter Highway as a Phase 1 project. It is recommended that this seven-lane cross section be extended west to Main Street to provide the needed east/west capacity on 12600 South through Mountain View Corridor.

The poor level of service during the evening peak hour at the Bangerter Highway / 12600 South intersection can be attributed to the need for additional eastbound capacity on the northbound offramp, particularly for the northbound left-turn movement. It is recommended that additional capacity be added for the northbound left-turn movement, as well as for the westbound through movement. It is recommended that an additional westbound lane through the interchange be added along with the planned improvements to 12600 South.

The poor level of service during the evening peak hour at the northbound Mountain View Corridor / 13400 South intersection can be attributed to the need for additional westbound capacity at the intersection. It is recommended that an additional westbound through lane be added to 13400 South between Mountain View Corridor and 5000 West. This would match the existing cross section that currently exists on 13400 South east of Mountain View Corridor.

Hales Engineering analyzed a mitigated scenario assuming that these recommended improvements had been implemented. It is anticipated that with these recommended improvements the Main Street / Herriman Boulevard / 12600 South, Mountain View Corridor / 12600 South, Bangerter Highway / 12600 South, and Mountain View Corridor / 13400 South intersections will operate at acceptable levels of service during the morning and evening peak hours.

No additional mitigation measures are recommended.



IV. FUTURE (2027) BACKGROUND CONDITIONS

A. Purpose

The purpose of the future (2027) background analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions. Through this analysis, future background traffic operational deficiencies can be identified, and potential mitigation measures recommended.

B. Roadway Network

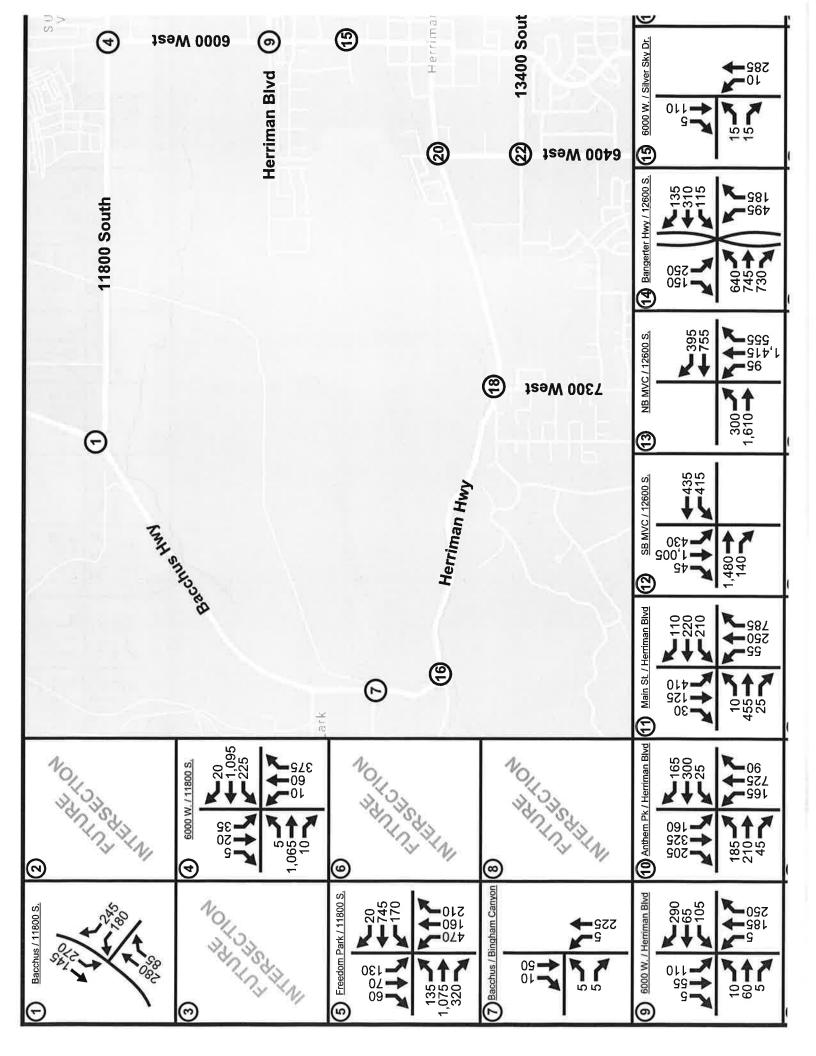
It was assumed that all previously recommended mitigation measures had been implemented prior to 2027. These mitigation measures include:

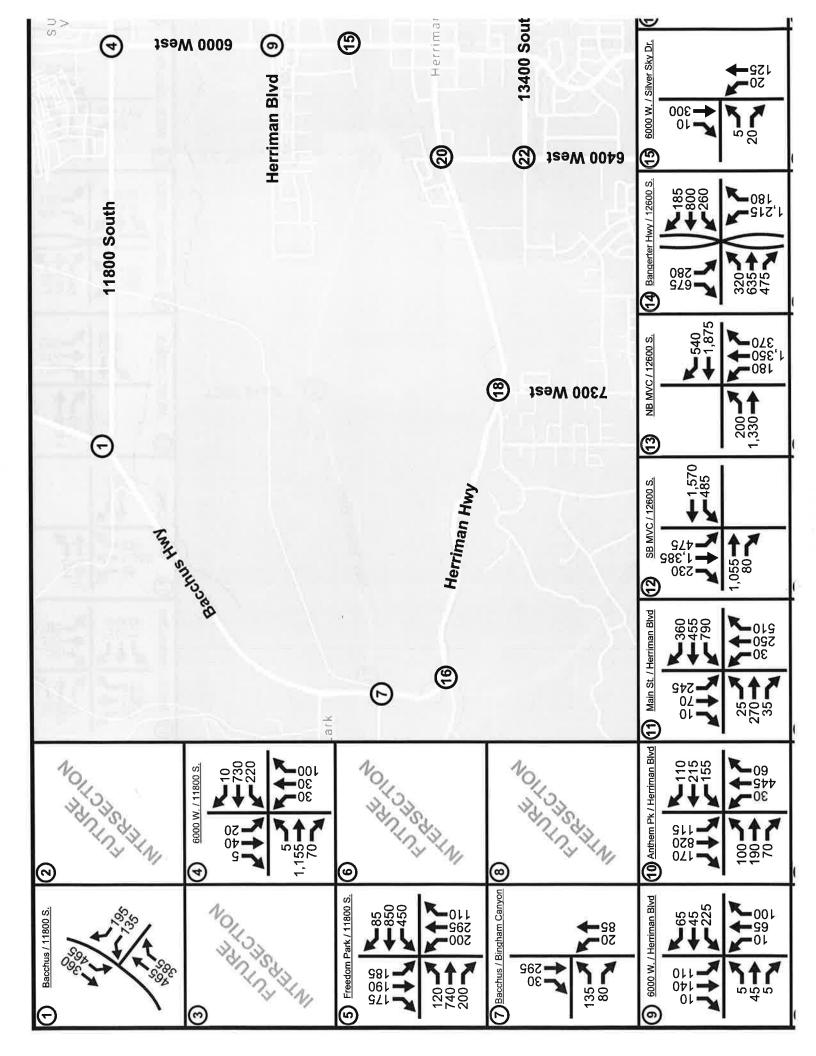
- Additional east/west travel lanes on 12600 South
- Additional east west travel lanes on 13400 South
- A channelized right-turn lane on the northbound approach to the Main Street / Herriman Boulevard / 12600 South intersection.
- Capacity improvements for the northbound left-turn movement at the Bangerter Highway / 12600 South interchange.

According to the WFRC Regional Transportation Plan, there are several additional improvement projects in the study area that are planned as Phase 1 (2019-2030) projects. However, none of these improvements were assumed to be completed prior to 2027.

C. Traffic Volumes

Hales Engineering obtained future (2027) forecasted volumes from a modified version of the WFRC / MAG travel demand model (TDM). This version of the WFRC/MAG TDM was tailored specifically for this project by Horrocks Engineers (and reviewed by Salt Lake County) to forecast future average weekday daily traffic (AWDT) volumes within the study area. Peak period turning movement counts were estimated using National Cooperative Highway Research Program (NCHRP) 255 methodologies which utilize existing peak period turn volumes and future AWDT volumes to project the future turn volumes at the major intersections. Future (2027) morning and evening peak hour turning movement volumes are shown in Figure 7 and Figure 8.







D. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at LOS E or LOS F in future (2027) background conditions as shown in Table 6 and Table 7:

- Bacchus Highway / 11800 South (Morning and Evening Peak)
- 6000 West / 11800 South (Morning and Evening Peak)
- Main Street / Herriman Boulevard / 12600 South (Morning Peak)
- SB Mountain View Corridor / 12600 South (Morning and Evening Peak)
- SB Mountain View Corridor / 13400 South (Morning Peak)
- 6400 West / Main Street (Evening Peak)
- 6400 West / 13400 South (Evening Peak)
- 5600 West / 13400 South (Evening Peak)
- NB Mountain View Corridor / 13400 South (Morning and Evening Peak)

These results serve as a baseline condition for the impact analysis of the proposed development for future (2027) conditions.

E. Queuing Analysis

Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - Southbound Approach 425 feet (p.m. peak)
 - Westbound Approach ->1,000 feet (a.m. and p.m. peak)
- 6000 West / 11800 South
 - Westbound Approach >1,000 feet (a.m. and p.m. peak)
- Freedom Park Drive / 11800 South
 - Northbound Approach 640 feet (a.m. peak)
 - Southbound Approach 640 feet (p.m. peak)
 - Eastbound Approach 435 feet (a.m. peak)
 - Westbound Approach 670 feet (p.m. peak)
- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach 815 feet (a.m. peak)
 - Southbound Approach 540 feet (p.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Northbound Approach 665 feet (a.m. peak)
 - Southbound Approach >1,000 feet (a.m. peak)
 - Westbound Approach 655 feet (p.m. peak)
- Mountain View Corridor / 12600 South



- Northbound Approach 485 feet (a.m. peak), 500 feet (p.m. peak)
- Southbound Approach 975 feet (p.m. peak)
- Eastbound Approach 680 feet (a.m. peak), 400 feet (p.m. peak)
- Westbound Approach 610 feet (a.m. peak), 960 feet (p.m. peak)
- Bangerter Highway / 12600 South
 - Northbound Offramp 690 feet (p.m. peak)
 - Southbound Offramp 405 feet (p.m. peak)
- 6400 West / Main Street
 - Northbound Approach 865 feet (p.m. peak)
 - Eastbound Approach 995 feet (p.m. peak)
 - Westbound Approach 945 feet (p.m. peak)
- 5600 West / Main Street
 - Southbound Approach 635 feet (p.m. peak)
- 6400 West / 13400 South
 - Southbound Approach >1,000 feet (p.m. peak)
- 5600 West / 13400 South
 - Northbound Approach 370 feet (a.m. peak), 360 feet (p.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
 - Eastbound Approach 470 feet (a.m. peak), 520 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 5000 West / 13400 South
 - Southbound Approach 630 feet (a.m. peak)
 - Eastbound Approach 870 feet (a.m. peak)
- Mountain View Corridor / 13400 South
 - Northbound Approach ->1,000 feet (a.m. and p.m. peak)
 - Southbound Approach 805 feet (p.m. peak)
 - Eastbound Approach 875 feet (a.m. peak)
 - Westbound Approach 610 feet (a.m. peak), 760 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.



Table 6: Future (2027) Background Morning Peak Hour Level of Service

Intersection		Wor	st Approach	- 1	Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	WB Stop	WB	>75.0	F	(e)	3.0	D (40.6)
6000 West / 11800 South	Signal				66.6	Ε	C (26.8)
Freedom Park Drive / 11800 South	Signal		1.00	•	35.5	D	
Bingham Canyon Mine / Bacchus Highway	EB Stop	ЕВ	3.2	A	(- :	•	-
6000 West / Herriman Boulevard	Signal	•	16	÷	8.3	A	-
Anthem Park Boulevard / Herriman Boulevard	Signal	i¥.	:⊭	-	30.3	С	12
Main Street / Herriman Boulevard / 12600 South	Signal	-		*	64.5	Ε	C (27.9)
SB MVC / 12600 South	Signal		28		70.4	Е	D (54.2)
NB MVC / 12600 South	Signal	3	•	ž	36.4	D	
Bangerter Highway / 12600 South	Signal	*			28.0	С	•
Silver Sky Drive / 6000 West	EB Stop	EB	4.3	Α	-	•	¥
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	EB	3.7	Α	(2)	(<u>s</u>	18
7300 West / Herriman Highway	NB Stop	NB	7.0	Α	逶	(e	•
6400 West / Main Street	NB/SB Stop	NB	29.1	D	S # 5	;•.	
5600 West / Main Street	Signal		(-	16.8	В	
6400 West / 13400 South	Signal	ě		Ē	16.2	В	Ť
5600 West / 13400 South	Signal	14	Se		25.7	С	'
5000 West / 13400 South	Signal	•		ş	44.7	D	
SB MVC / 13400 South	Signal		3#:		63.1	Е	C (22.1)
NB MVC / 13400 South	Signal		1		62.6	Ε	B (18.3)

Source: Hales Engineering, October 2019

This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.
 This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.
 SB = Southbound approach, etc.



Table 7: Future (2027) Background Evening Peak Hour Level of Service

Intersection		Worst Approach Overall Intersect					on Mitigated	
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS²	LOS (Delay)	
Bacchus Highway / 11800 South	WB Stop	WB	>75.0	F		3.00	D (45.7)	
6000 West / 11800 South	Signal	<u>3€</u>		•	73.1	Е	C (21.3)	
Freedom Park Drive / 11800 South	Signal	:s -	-	> € 5	46.7	D		
Bingham Canyon Mine / Bacchus Highway	EB Stop	EB	5.5	Α	120	*	*	
6000 West / Herriman Boulevard	Signal	n is	*		9.3	A	*	
Anthem Park Boulevard / Herriman Boulevard	Signal	ηğ	¥	Œ.	19.6	В	2	
Main Street / Herriman Boulevard / 12600 South	Signal	¥	æ	343	36.3	D	; = :	
SB MVC / 12600 South	Signal	¥		140	56.0	E	D (43.4)	
NB MVC / 12600 South	Signal	8		252	46.0	D		
Bangerter Highway / 12600 South	Signal	-	84	-	42.9	D	125	
Silver Sky Drive / 6000 West	EB Stop	EB	3.8	Α			82	
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	ЕВ	4.3	A	(#)	٠	::	
7300 West / Herriman Highway	NB Stop	NB	8.3	Α	98	:::	5 7 5	
6400 West / Main Street	NB/SB Stop	NB	>120.0	F	*		B (17.8)	
5600 West / Main Street	Signal	¥	N=	1/2	22.1	С	85	
6400 West / 13400 South	Signal	*			98.8	F	B (17.1)	
5600 West / 13400 South	Signal	¥	1/2=	0.5	89.9	F	D (54.9)	
5000 West / 13400 South	Signal	*	0mi	(±)	20.4	С	6,≅	
SB MVC / 13400 South	Signal	*	V.=	V.	37.4	D	72	
NB MVC / 13400 South	Signal		59.	, j	70.7	E	D (38.0)	

1. This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.

Source: Hales Engineering, October 2019

F. Mitigation Measures

It is anticipated that by 2027 traffic volumes at the Bacchus Highway / 11800 South intersection will warrant the installation of a traffic signal (based on Utah MUTCD 2009 Chapter 4C Warrant



3). It is also anticipated that dual left-turn lanes will be warranted on the southbound approach based on the projected evening peak hour volumes. It is recommended that this intersection be monitored and that the improvements be implemented when warrants are met.

The poor level of service at the 6000 West / 11800 South intersection can be attributed to large delays experienced by westbound left-turning vehicles. It is recommended that the signal cycle length at this intersection be increased to 120 seconds, that permitted/protected left-turn phasing be added for the westbound approach, and that the westbound left-turn storage lane length be increased.

It is anticipated that dual left-turn lanes will be warranted on the southbound approach to the Main Street / Herriman Boulevard / 12600 South intersection based on projected morning peak hour volumes. It is recommended that this improvement be completed prior to 2027 and that the signal cycle length at this intersection be increased to 120 seconds during the morning peak hour (the evening peak hour cycle length is already set to 120 seconds).

It is anticipated that additional capacity will be needed at the Mountain View Corridor / 12600 South intersection. It is recommended that dual left-turn lanes be installed for the south- and westbound movements, and that the signal cycle length at this intersection be increased to 120 seconds during the morning peak hour (the evening peak hour cycle length is already set to 120 seconds).

It is anticipated that additional capacity will be needed at the 6400 West / Main Street intersection to accommodate the projected 2027 traffic volumes, particularly during the evening peak hour. It is recommended that a separate right-turn lane be added to the northbound approach and that separate left-turn lanes be added to the east- and westbound approaches. This would allow left-turning vehicles to queue and wait for gaps without blocking other movements.

The south- and eastbound approaches to the 6400 West / 13400 South intersection currently consist of a single lane. It is recommended that these approaches be expanded to match the north- and westbound approaches which consist of a through lane with separate left- and right-turn lanes.

It is anticipated that dual left-turn lanes will be warranted on the south- and westbound approaches to the 5600 West / 13400 South intersection based on projected 2027 evening peak hour traffic projections.

It is recommended that the signal cycle length at the 5000 West / 13400 south intersection be increased to 120 seconds during the morning peak hour (the evening peak hour cycle length is already set to 120 seconds), and that the signal be coordinated with other signals on the 13400 South corridor



It is anticipated that additional capacity will be needed at the Mountain View Corridor (SR-85) / 13400 South intersection to accommodate the projected 2027 traffic volumes. According to the WFRC RTP, Mountain View Corridor south of 13400 South is planned to be widened to three lanes in each direction prior to 2030.

Hales Engineering analyzed a mitigated scenario assuming that these recommended improvements had been implemented. It is anticipated that with these recommended improvements the Bacchus Highway / 11800 South intersection will improve to LOS D during the morning and evening peak hours and the 6000 West / 11800 South intersection will improve to LOS C during the morning and evening peak hours. It is anticipated that the Main Street / Herriman Boulevard / 12600 South intersection will improve to LOS C during the morning peak hour. It is also anticipated that with the recommended improvements the southbound Mountain View Corridor / 12600 South intersection will improve to LOS D during the morning and evening peak hours.

With the previously recommended mitigation measures the poor levels of service at the 6400 West / Main Street and 5600 West / 13400 South intersections are anticipated to persist during the evening peak hour, and the poor levels of service at the Mountain View Corridor / 13400 South intersections are anticipated to persist in the morning and evening peak hours.

The projected evening peak hour traffic volumes at the 6400 West / Main Street intersection will warrant the installation of a traffic signal (based on Utah MUTCD 2009 Chapter 4C Warrant 3). Therefore, a traffic signal is recommended at this intersection.

Despite the addition of dual left-turn lanes to the 5600 West / 13400 South intersection, it is anticipated that additional capacity will still be needed at the intersection during the evening peak hour. It is recommended that an additional southbound through lane be added to the intersection to increase capacity.

As previously discussed, the Mountain View Corridor is planned to have a freeway facility constructed in the median area to create a freeway/frontage road system with segments planned to be constructed in either Phase 2 (2031-2040) or Phase 3 (2041-2050). It is also recommended that the northbound right-turn movement be a channelized free right-turn movement.

An additional mitigated scenario was analyzed assuming that freeway lanes had been constructed on Mountain View Corridor creating a grade separated intersection at 13400 South. It was assumed that 75% of north- and southbound through traffic would be carried by the freeway lanes with 25% remaining on the frontage roads. (This assumption is based on the projected ratio of ADTs on the freeway and frontage roads segments in the 2042 travel demand model runs that were developed for this study.) This scenario also assumed that an additional southbound through lane had been added to the 5600 West / 13400 South intersection, and a channelized free right-



turn lane had been added to northbound Mountain View Corridor at 13400 South. With these additional mitigation measures, all study intersections are anticipated to operate at acceptable levels of service.

No additional mitigation measures are recommended.



V. PROJECT CONDITIONS

A. Purpose

The project conditions discussion explains the type and intensity of development. This provides the basis for trip generation, distribution, and assignment of project trips to the surrounding study intersections defined in Chapter I.

B. Project Description

The proposed Olympia Hills development located generally between 6400 West and Bacchus Highway on the east and west, and 12600 South and Herriman Highway on the north and south. The development will consist of several land uses in a mixed-use setting. Metrostudy completed an analysis of the project area to determine appropriate land use types, absorption rates, and build timelines for Olympia Hills.

Based on the Metrostudy analysis, IBI Group developed a land use plan with unit counts and building sizes by area. The project will consist of four town / village centers with higher density and other areas with lower density. The project is being proposed to be built in four five-year phases with the first phase being completed in 2027. A concept and phasing plan for the proposed development is provided in Appendix B.

The proposed land use for Phase I (2027) has been identified as follows:

•	Single-family detached housing	219 Units
•	Multi-family housing	1,223 Units
•	Commercial/Retail	150,000 sq. ft.
•	Office Buildings	638,500 sq. ft.

Note: Phase 1 includes half of Village Center C and half of the Town Center.

The additional proposed land use for Phase II (2032) has been identified as follows:

•	Single-family detached housing	516 Units
•	Multi-family housing	1,379 Units
•	Commercial/Retail	172,000 sq. ft.
•	Office Buildings	698,200 sq. ft.

Note: Phase 2 includes half of Village Center C, half of the Town Center, and half of Village Center A.



The additional proposed land use for Phase III (2037) has been identified as follows:

Single-family detached housing
 Multi-family housing
 Commercial/Retail
 Office Buildings
 125 Units
 59,000 sq. ft.
 57,300 sq. ft.

Note: Phase 3 includes half of Village Center A and all of Village Center B.

The additional proposed land use for Phase IV (2042) has been identified as follows:

• Single-family detached housing 90 Units

Multi-family housing
 1,109 Units

In summary, the proposed land use for all of Olympia Hills has been identified as follows:

Single-family detached housing
Multi-family housing
Commercial/Retail
Office Buildings
950 Units
5,380 Units
381,000 sq. ft.
1,394,000 sq. ft.

C. Trip Generation

Trip generation for the development was calculated using trip generation rates published in the Institute of Transportation Engineers (ITE) *Trip Generation*, 10th Edition, 2017. Based on discussions with Salt Lake County and the development team, Hales Engineering also took trip reductions due to internal capture and transit use. Detailed trip generation tables are provided in Appendix C.

Internal capture rates were calculated for the Town Center and the Village Centers using standard ITE methodologies discussed in the ITE *Trip Generation Handbook*, 3rd Edition, 2017 and NCHRP Report 684. Hales Engineering used the NCHRP 684 Internal Trip Capture Estimation Tool, which follows these methodologies. Detailed internal capture calculations are shown in Appendix C.

Trip reductions due to transit use were determined based on transit ridership in neighboring communities and the anticipated transit types that may be available in the Olympia Hills development. The following transit data were pulled from the 2017 American Community Survey (formerly known as Journey to Work):

Riverton: 2.5%
 South Jordan: 3.2%
 West Jordan: 2.3%
 Herriman: 1.1%



It is anticipated that Olympia Hills will be more conducive to transit ridership than the surrounding communities due to the concentrated densities of the town and village centers. It is also anticipated that the types of transit that will be available will be similar to that of Riverton. Therefore, a 2.5% transit reduction, which is equal to the Riverton transit ridership, was assumed. It was assumed that Olympia Hills would have access to transit by Phase II (2032).

A summary of the trip generation after reductions for Olympia Hills is included in Table 8.

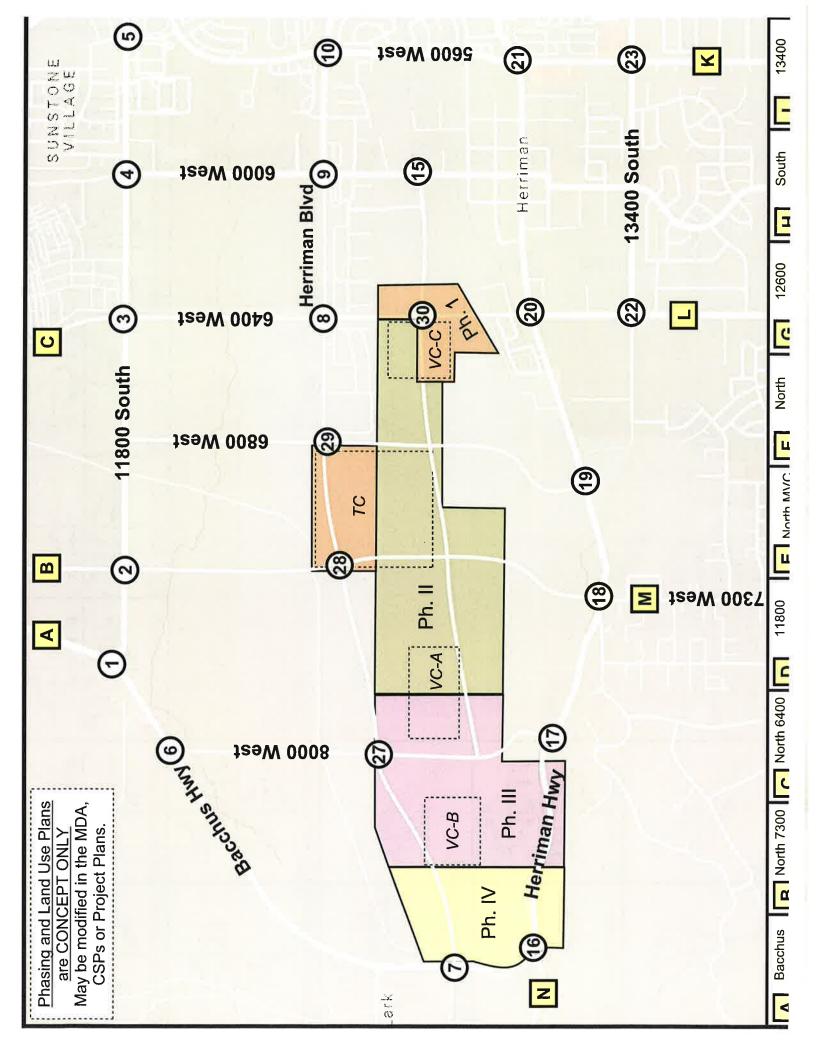
Reduced Trips Phase Time In Out Total 23,544 11,772 11,772 Daily Phase I 755 663 1,418 AM (2027)PM844 1,060 1,904 Daily 25,414 25,414 50,828 Phase II 1,574 3,062 AM 1,488 (2032)PM 1,855 2,222 4,077 Daily 33,563 33,563 67,126 Phase III 1,830 2,114 3,944 AM (2037)PM2,502 2,692 5.194 Daily 38,091 38,091 76,182 Phase IV AM 1,953 2,519 4,472 (2042)PM2,869 5,775 2,906

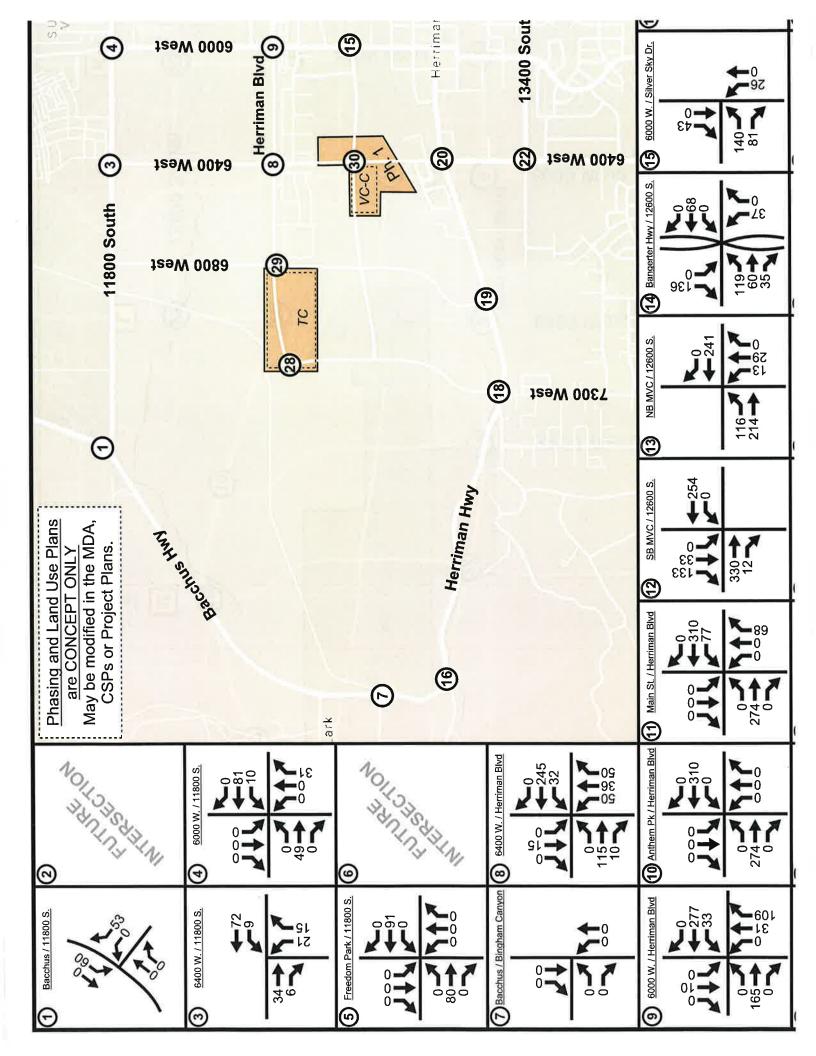
Table 8: Trip Generation Summary

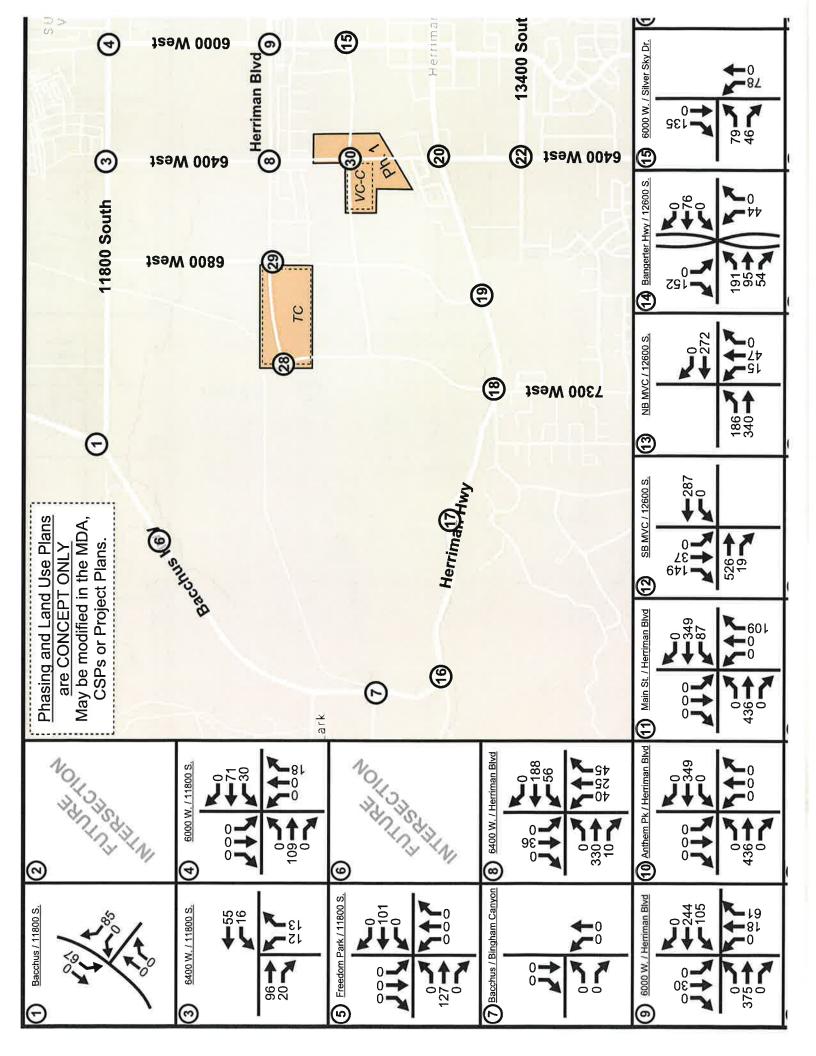
D. Trip Distribution and Assignment

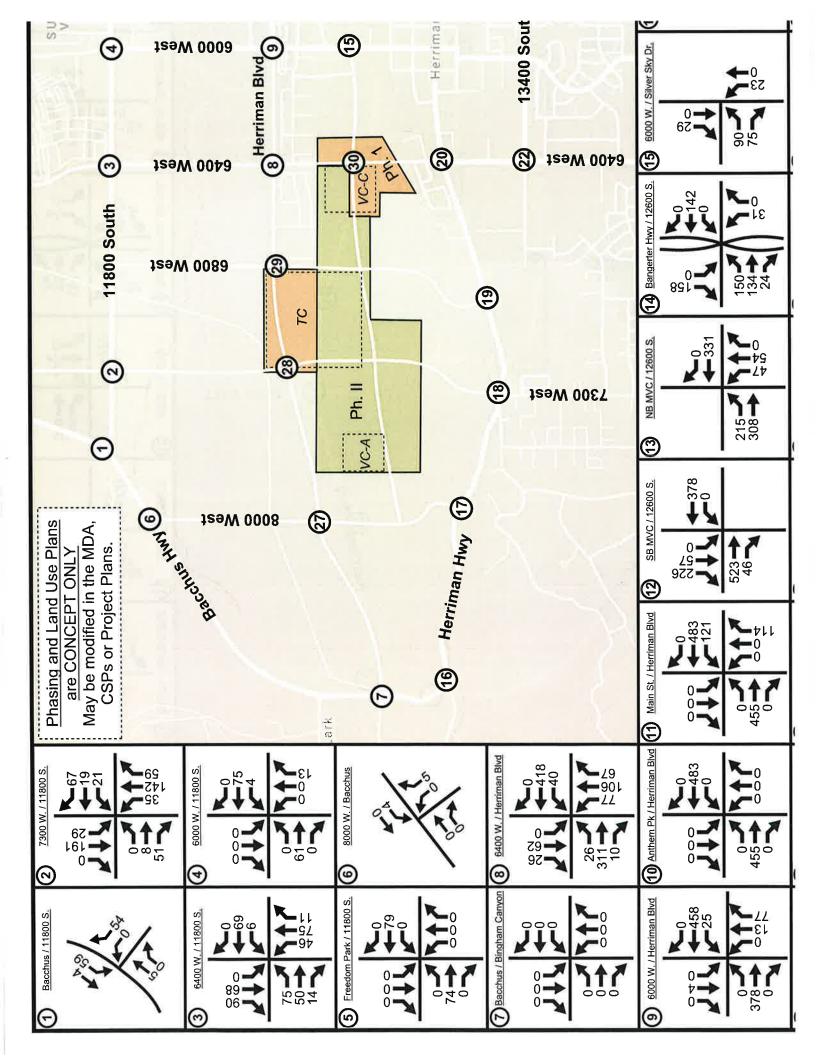
Trip distribution for the Olympia Hills project was developed based on a select link analysis in the build travel demand models of the project. Horrocks Engineers ran the analysis, which provided the distribution of project trips in the study network. The distribution percentages of project trips entering and exiting 14 separate external nodes were calculated based on the select link analysis results. A summary of the assumed trip distribution based on the select link analysis is shown in Figure 9.

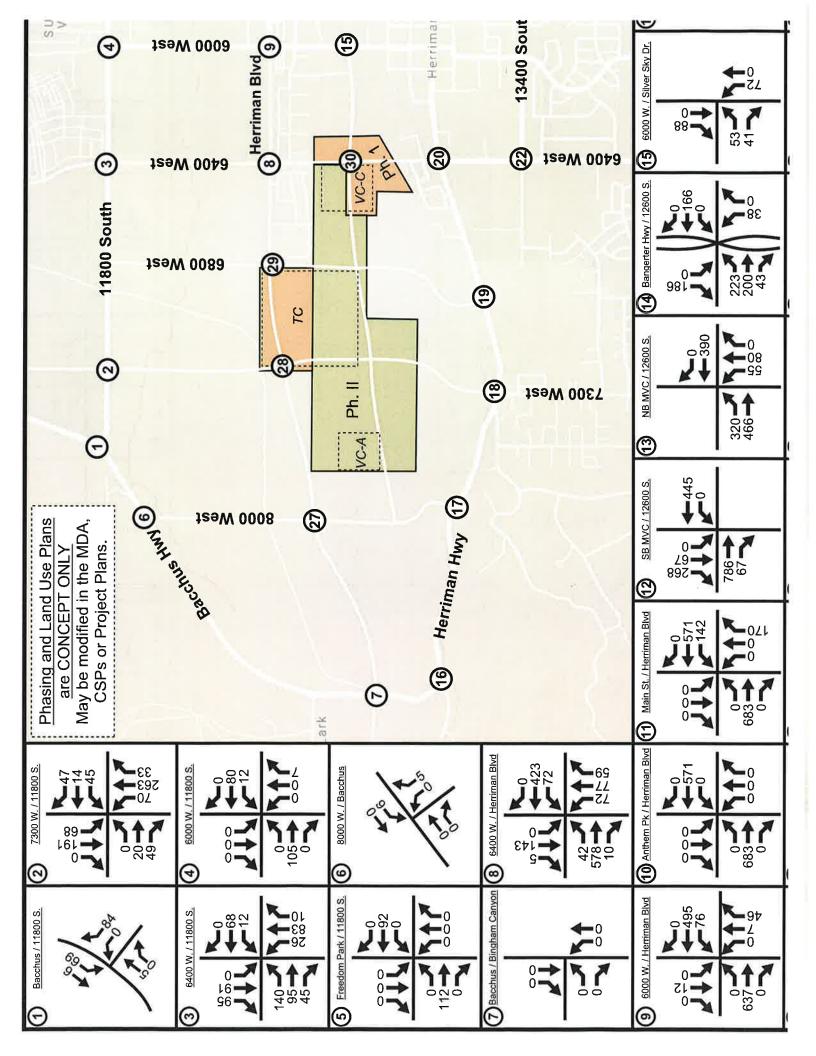
These trip distribution assumptions were used to assign the morning and evening peak hour trip generation at the study intersections to create trip assignment for the proposed development. The detailed select link results along each route were used as a guide to assign trips to the appropriate routes. Trip assignment volumes for the development for each phase and peak hour are shown in Figures 10 through 17.

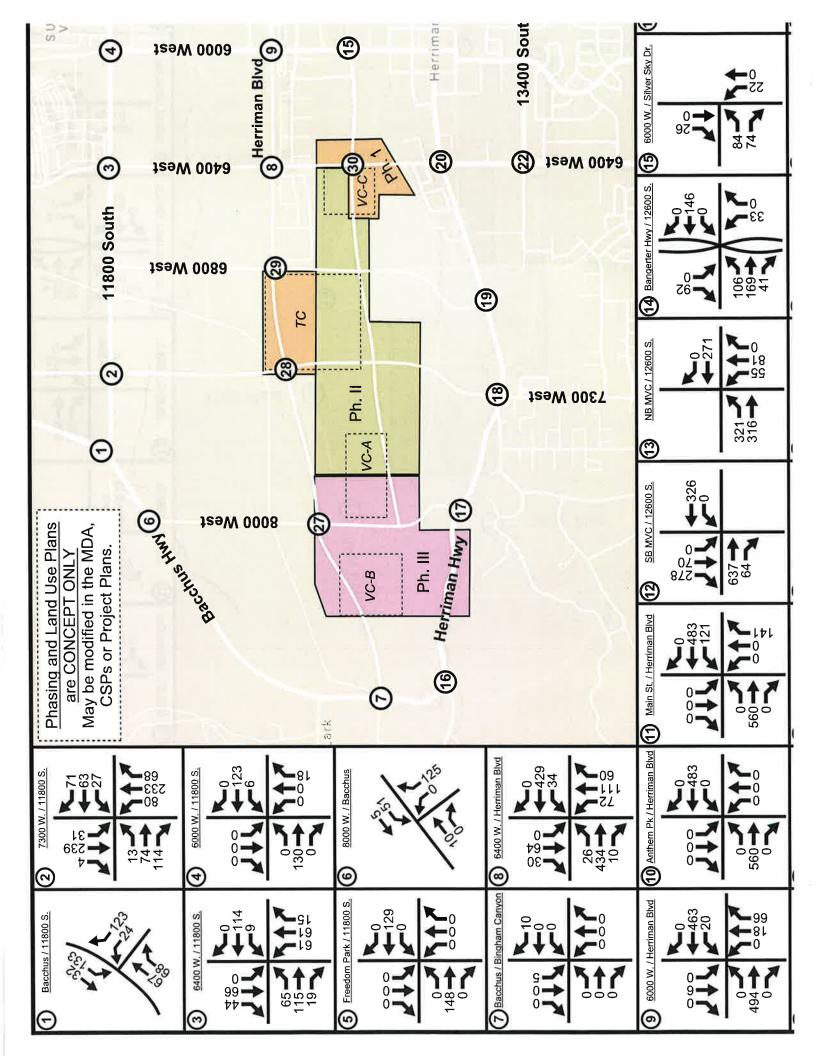


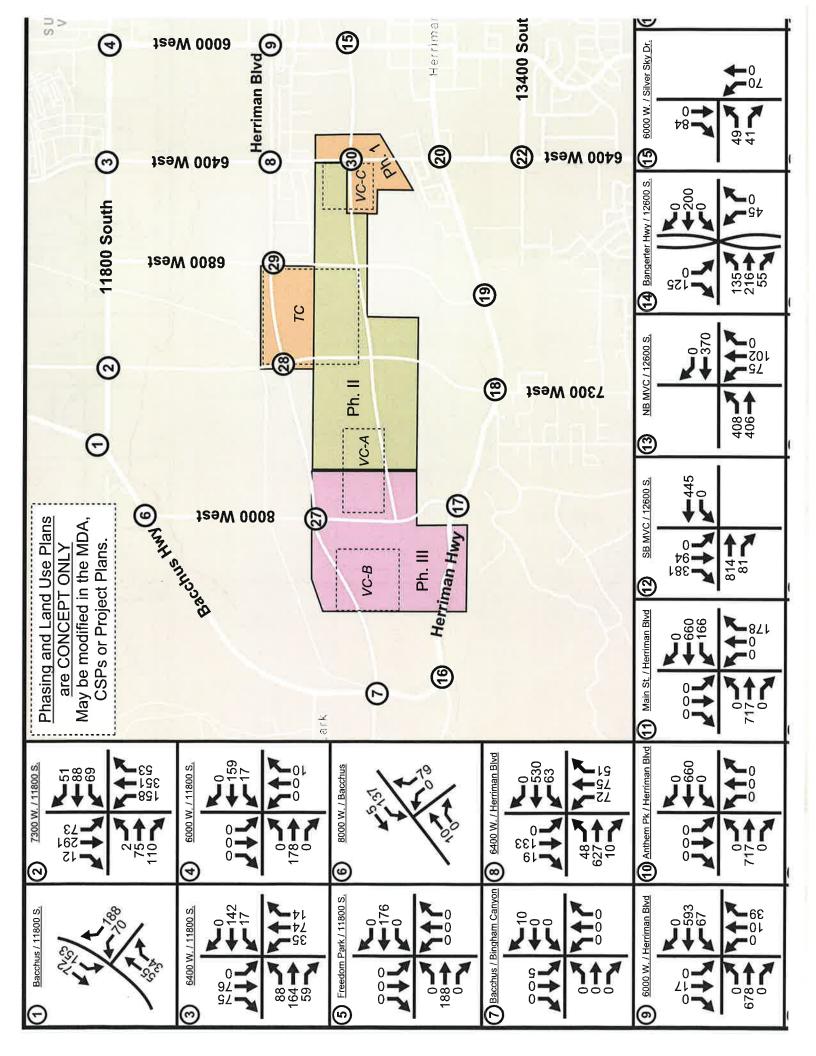


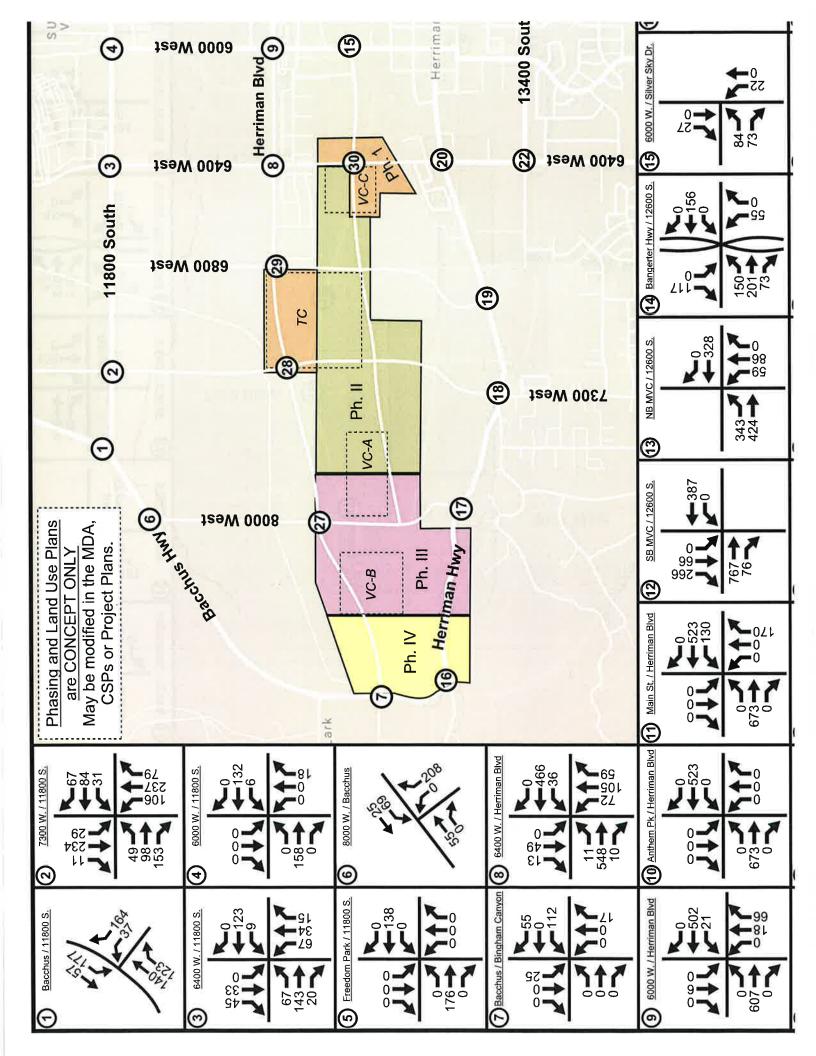


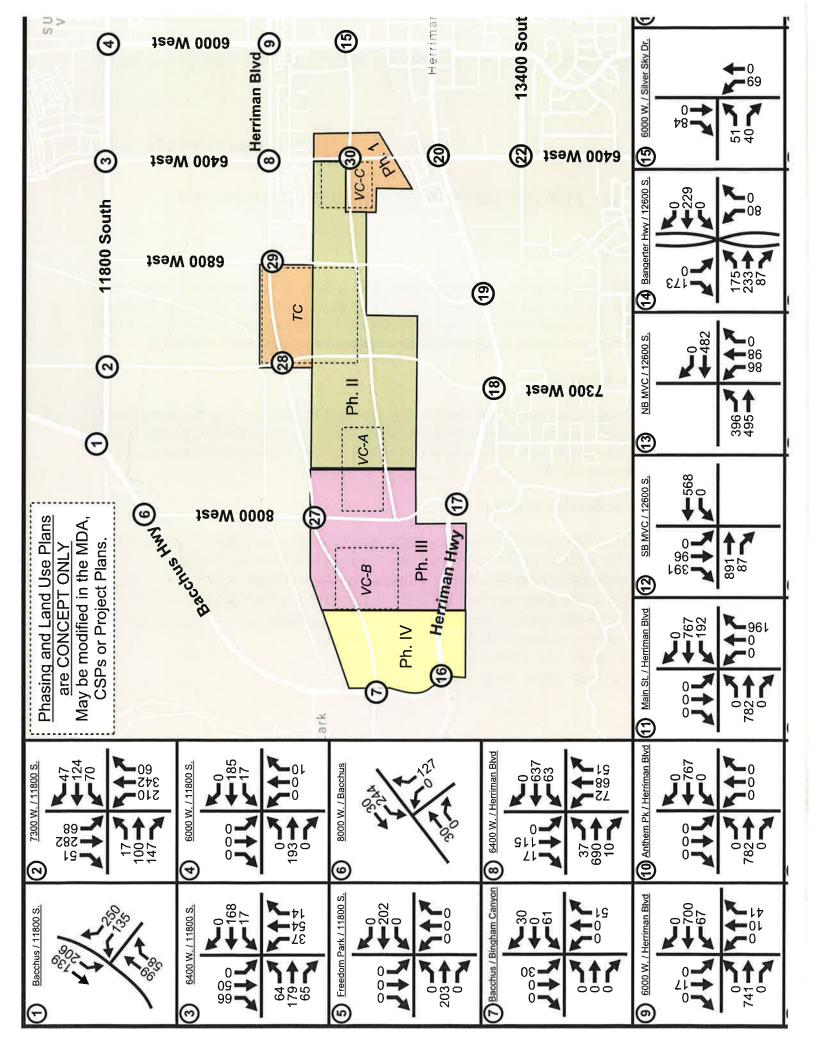














VI. FUTURE (2027) PLUS PROJECT CONDITIONS

A. Purpose

The purpose of the future (2027) plus project analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions plus the net trips generated by the proposed development. This scenario provides valuable insight into the potential impacts of the proposed project on future background traffic conditions.

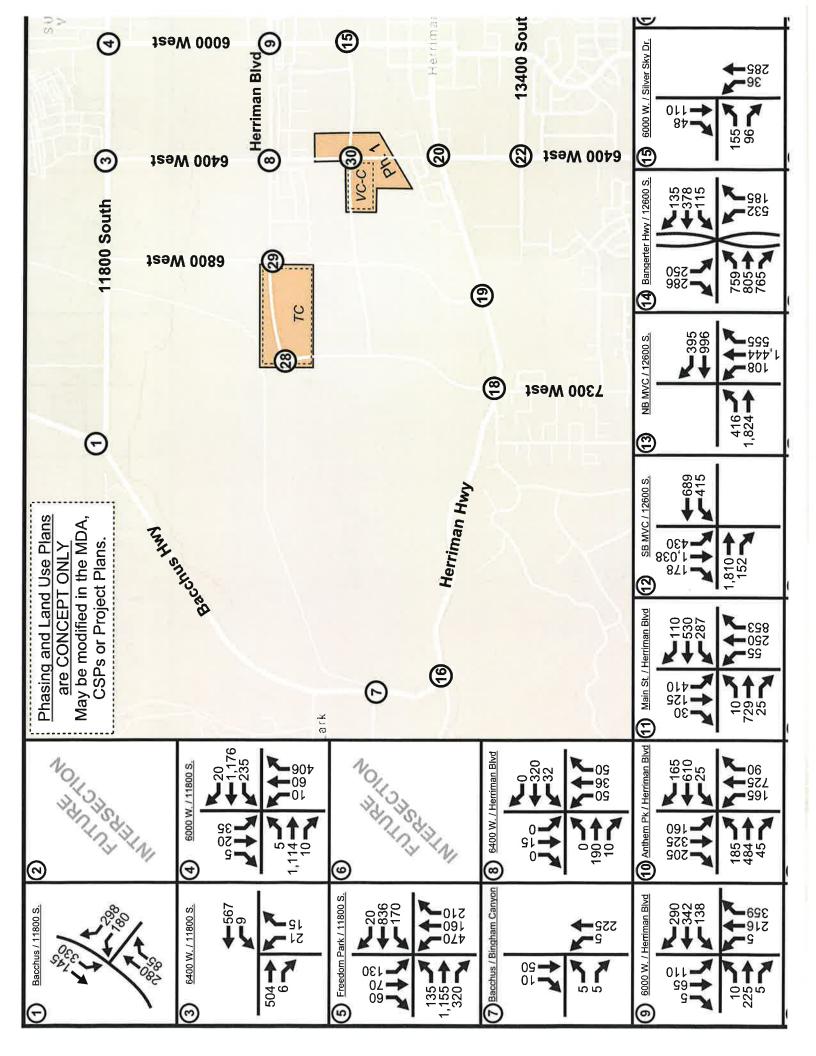
B. Traffic Volumes

Hales Engineering added the Phase I project trips discussed in Chapter V to the future (2027) background traffic volumes to predict turning movement volumes for future (2027) plus project conditions. Future (2027) plus project evening peak hour turning movement volumes are shown in Figure 18 and Figure 19.

C. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at LOS E or LOS F in future (2027) plus project conditions as shown in Table 9 and Table 10:

- Anthem Park Boulevard / Herriman Boulevard (Morning Peak)
- SB Mountain View Corridor / 12600 South (Morning and Evening Peak)
- NB Mountain View Corridor / 12600 South (Evening Peak)
- Bangerter Highway / 12600 South (Evening Peak)
- 6400 West / Main Street (Evening Peak)
- 5600 West / 13400 South (Evening Peak)



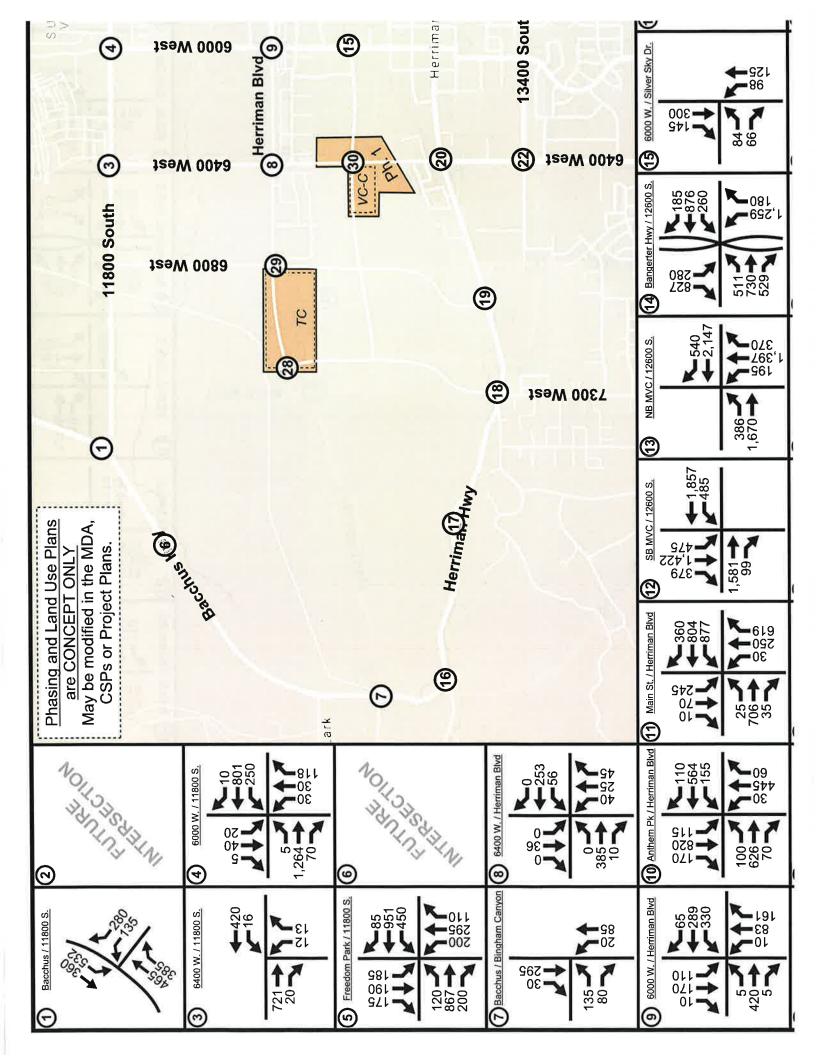




Table 9: Future (2027) Plus Project Morning Peak Hour Level of Service

Intersection		Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal		18		41.6	D	
6400 West / 11800 South	NB Stop	NB	14.5	В	2		
6000 West / 11800 South	Signal	•	<u>, e</u>	•	32.8	С	•
Freedom Park Drive / 11800 South	Signal	N=8	(#)	*	33.6	С	
Bingham Canyon Mine / Bacchus Highway	EB Stop	ЕВ	3.1	A	¥	786	340
6400 West / Herriman Boulevard	SB Stop	SB	8.2	A	·		- F
6000 West / Herriman Boulevard	Signal	÷	<u></u>	*	12.9	В	
Anthem Park Boulevard / Herriman Boulevard	Signal	(2)	196	147	63.8	E	D (44.6)
Main Street / Herriman Boulevard / 12600 South	Signal	:•:	s e .	٠	43.5	D	3%
SB MVC / 12600 South	Signal	*	œ.		60.6	Е	D (38.6)
NB MVC / 12600 South	Signal	(*)	:(+ :	9 0 0	35.9	D	<u> </u>
Bangerter Highway / 12600 South	Signal	(<u>=</u> 1)	(-)		29.2	С	
Silver Sky Drive / 6000 West	EB Stop	EB	8.5	Α	*	855	
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	ЕВ	4.0	Α	5.	٠	· .
7300 West / Herriman Highway	NB/SB Stop	SB	8.2	A	¥	-	540
6800 West / Herriman Highway	SB Stop	SB	7.5	Α	¥	125	*
6400 West / Main Street	Signal		8 2 :	***	27.1	С	48
5600 West / Main Street	Signal	~ (S	Y-8	3 9	18.4	В	<u> </u>
6400 West / 13400 South	Signal	8	3		16.7	В	
5600 West / 13400 South	Signal	360	.*	340	34.2	С	
5000 West / 13400 South	Signal	@r	•	*	33.0	С	2
SB MVC / 13400 South	Signal	: €\		* /	26.7	С	5401
NB MVC / 13400 South	Signal	(¥)	92	₩.	19.6	В	- FE
6800 West / Herriman Boulevard	NB/SB Stop	SB	6.2	Α	E .	8	(a)
Silver Sky Drive / 6400 West	EB/WB Stop	ЕВ	4.3	A	ĕ	76E	(A)

^{1.}This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalised intersections. A.This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections A.SB = Southbound approach, etc.

Source: Hales Engineering, November 2019



Table 10: Future (2027) Plus Project Evening Peak Hour Level of Service

Intersection	-0-	Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS1	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal	¥I	(=)(*	46.3	D	
6400 West / 11800 South	NB Stop	NB	13.4	В	8\$1		
6000 West / 11800 South	Signal	2.	147	-	36.7	D	
Freedom Park Drive / 11800 South	Signal	- 5	:5:	18	50.6	D	<u> </u>
Bingham Canyon Mine / Bacchus Highway	EB Stop	ЕВ	5.9	Α	*	270	
6400 West / Herriman Boulevard	SB Stop	SB	10.9	В		(7)	-
6000 West / Herriman Boulevard	Signal	(4);	(#K)		16.3	В	-
Anthem Park Boulevard / Herriman Boulevard	Signal	(20)	æ		26.7	С	
Main Street / Herriman Boulevard / 12600 South	Signal	æ:	٠	-	36.0	D	•
SB MVC / 12600 South	Signal	**	*		71.1	E	C (26.7)
NB MVC / 12600 South	Signal	3	•		108.1	F	C (23.7)
Bangerter Highway / 12600 South	Signal	. ₩(.	> .		96.2	F	D (41.9)
Silver Sky Drive / 6000 West	EB Stop	EB	8.9	Α			26
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	ЕВ	5.4	A	-	:•:	*)
7300 West / Herriman Highway	NB/SB Stop	SB	14.5	В	25		¥
6800 West / Herriman Highway	SB Stop	SB	13.2	В	/=	*	•
6400 West / Main Street	Signal	*	*	(*)	80.4	F	C (30.2)
5600 West / Main Street	Signal	1	•	2	22.0	С	*
6400 West / 13400 South	Signal		55	<u></u>	19.3	В	 8
5600 West / 13400 South	Signal	*			70.7	Ε	D (52.5)
5000 West / 13400 South	Signal		150	£ 7 /,	23.0	С	÷1
SB MVC / 13400 South	Signal	760	(®	(* €)	23.3	С	*
NB MVC / 13400 South	Signal		©	-	49.5	D	
6800 West / Herriman Boulevard	NB/SB Stop	NB	8.2	A		۹	
Silver Sky Drive / 6400 West	EB/WB Stop	ЕВ	4.2	Α	2	-	4/

^{1.} This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections

3. SB = Southbound approach, etc.

Source: Hales Engineering, November 2019



D. Queuing Analysis

Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - Northbound Approach 615 feet (a.m. peak), 595 feet (p.m. peak)
- 6000 West / 11800 South
 - Northbound Approach 400 feet (a.m. peak)
 - Eastbound Approach >1,000 feet (a.m. and p.m. peak)
- Freedom Park Drive / 11800 South
 - Northbound Approach 610 feet (a.m. peak), 510 feet (p.m. peak)
 - Southbound Approach 650 feet (p.m. peak)
 - Eastbound Approach 450 feet (a.m. peak)
 - Westbound Approach 820 feet (p.m. peak)
- · Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach >1,000 feet (a.m. peak)
 - Southbound Approach 615 feet (p.m. peak)
 - Eastbound Approach 560 feet (a.m. peak)
 - Westbound Approach 710 feet (a.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Northbound Approach 530 feet (a.m. peak)
 - Southbound Approach 905 feet (a.m. peak)
 - Eastbound Approach 400 feet (p.m. peak)
 - Westbound Approach 400 feet (p.m. peak)
- Mountain View Corridor / 12600 South
 - Northbound Approach 630 feet (a.m. peak), 540 feet (p.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach 475 feet (a.m. peak), >1,000 feet (p.m. peak)
- Bangerter Highway / 12600 South
 - Northbound Offramp >1,000 feet (p.m. peak)
 - Southbound Offramp 790 feet (p.m. peak)
 - Westbound Approach 875 feet (p.m. peak)
- 6400 West / Main Street
 - Westbound Approach 950 feet (p.m. peak)
- 5600 West / Main Street
 - Southbound Approach 450 feet (p.m. peak)
- 5600 West / 13400 South
 - Northbound Approach 425 feet (a.m. peak)
 - Southbound Approach 690 feet (p.m. peak)



- Eastbound Approach 590 feet (a.m. peak), 670 feet (p.m. peak)
- Westbound Approach >1,000 feet (p.m. peak)
- 5000 West / 13400 South
 - Northbound Approach 475 feet (a.m. peak)
 - Eastbound Approach 475 feet (a.m. peak), 350 feet (p.m. peak)
- Mountain View Corridor / 13400 South
 - Southbound Approach 450 feet (p.m. peak)
 - Westbound Approach 780 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.

E. Mitigation Measures

It is recommended that a dedicated right-turn pocket be added to the westbound approach of the Anthem Park Boulevard / Herriman Boulevard intersection to increase the westbound capacity at the intersection. It is also recommended that the storage length of all left-turn lanes be increased at the intersection.

It is anticipated that dual left-turn lanes will be warranted at the Freedom Park Drive / 11800 South intersection on the westbound approach. It is recommended that westbound dual left-turn lanes be installed when warranted. It is also recommended that the cycle length of the signal be increased to 120 seconds.

It is anticipated that left-turn permissive-protected phasing will be warranted at the 6400 West / Main Street intersection on the westbound approach. It is recommended that this phasing be implemented when warranted. It is also recommended that a separate right-turn lane be installed on the eastbound approach of the intersection.

The delays at the 5600 West / 13400 South intersection can be attributed to lack of capacity at the intersection due to high westbound volumes during the evening peak hour. It is recommended that the storage length of all left- and right-turn lanes be increased, that a right-turn overlap phase be implemented on the westbound approach, and that the northbound right-turn lane be converted into a shared through-right lane.

Significant delays are anticipated at the Mountain View Corridor / 12600 South intersections in future (2027) plus project conditions. Ultimately, this section of Mountain View Corridor will include a grade-separated freeway corridor that will pull northbound and southbound through traffic off the frontage roads. This project is planned by WFRC to be completed by 2040. It is recommended that this freeway project be expedited to be built as soon as possible. In the meantime, the following mitigation measures can be implemented at the Mountain View Corridor / 12600 South intersection to reduce delays: an additional westbound through lane at the NB MVC / 12600 South



intersection and channelizing eastbound and westbound right-turns. It is anticipated that these improvements will also improve the performance at the Bangerter Highway / 12600 South intersection, as westbound queues from Mountain View Corridor were reaching Bangerter Highway previously.

With added capacity and throughput at the Mountain View Corridor / 12600 South intersections, it is anticipated that the westbound left-turn queue at the Main Street / Herriman Boulevard intersection will increase to where it interferes with Mountain View Corridor. It is recommended that westbound dual left turns be installed at the Main Street / Herriman Boulevard intersection when warranted.

Hales Engineering completed a mitigated scenario with the proposed improvements, including the Mountain View Corridor freeway. As done previously, it was assumed that approximately 25% of the northbound and southbound traffic will remain on the frontage roads when the freeway is built. Based on the mitigated scenario, is anticipated that the proposed improvements will improve the LOS at all study intersections to an acceptable level of service.



VII. FUTURE (2032) BACKGROUND CONDITIONS

A. Purpose

The purpose of the future (2032) background analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions. Through this analysis, future background traffic operational deficiencies can be identified, and potential mitigation measures recommended.

B. Roadway Network

It was assumed that all previously recommended background mitigation measures had been implemented prior to 2032. It was also assumed that all Phase 1 (2019-2030) improvements outlined in the WFRC RTP had been implemented These improvements include:

- 11800 South widened to a five-lane cross section between Bacchus Highway and 6000 West.
- Herriman Boulevard extended west to Bacchus Highway as a three-lane cross section.
- Herriman Highway/Main Street widened to a three-lane cross section between 7300
 West and 6200 West.
- 7300 West extended north from Herriman Highway to Herriman Boulevard as a threelane cross section.
- 6400 West extended north from Main Street to 10400 South as a three-lane cross section.
- 6000 West widened to a five-lane cross section between Main Street and Herriman Boulevard.

In addition to these improvements listed in the WFRC RTP, the 2030 WFRC/MAG TDM assumes that 7300 West had been extended farther north than Herriman Boulevard connecting to Bacchus Highway at a point north of 11000 South as a three-lane cross section. It was assumed that this improvement had been completed prior to 2032.

Also, it was assumed that Herriman Boulevard would be striped and widened to a five-lane cross-section between 6000 West and 6800 West by 2032, as the current pavement along much of this section is already 80 feet wide.

C. Traffic Volumes

Hales Engineering obtained future (2032) forecasted volumes from a modified version of the WFRC / MAG TDM. This version of the WFRC/MAG TDM was tailored specifically for this project



by Horrocks Engineers (and reviewed by Salt Lake County) to forecast future average weekday daily traffic (AWDT) volumes within the study area. Peak period turning movement counts were estimated using National Cooperative Highway Research Program (NCHRP) 255 methodologies which utilize existing peak period turn volumes and future AWDT volumes to project the future turn volumes at the major intersections. Future (2032) morning and evening peak hour turning movement volumes are shown in Figure 20 and Figure 21.

D. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at LOS E or LOS F in future (2032) background conditions as shown in Table 11 and Table 12:

- 6400 West / 11800 South (Morning and Evening Peak)
- SB Mountain View Corridor / 12600 South (Morning and Evening Peak)
- NB Mountain View Corridor / 12600 South (Evening Peak)
- SB Mountain View Corridor / 13400 South (Morning Peak)
- 5000 West / 13400 South (Morning Peak)
- SB Mountain View Corridor / 13400 South (Evening Peak)
- NB Mountain View Corridor / 13400 South (Morning and Evening Peak)

These results serve as a baseline condition for the impact analysis of the proposed development for future (2032) conditions.

E. Queuing Analysis

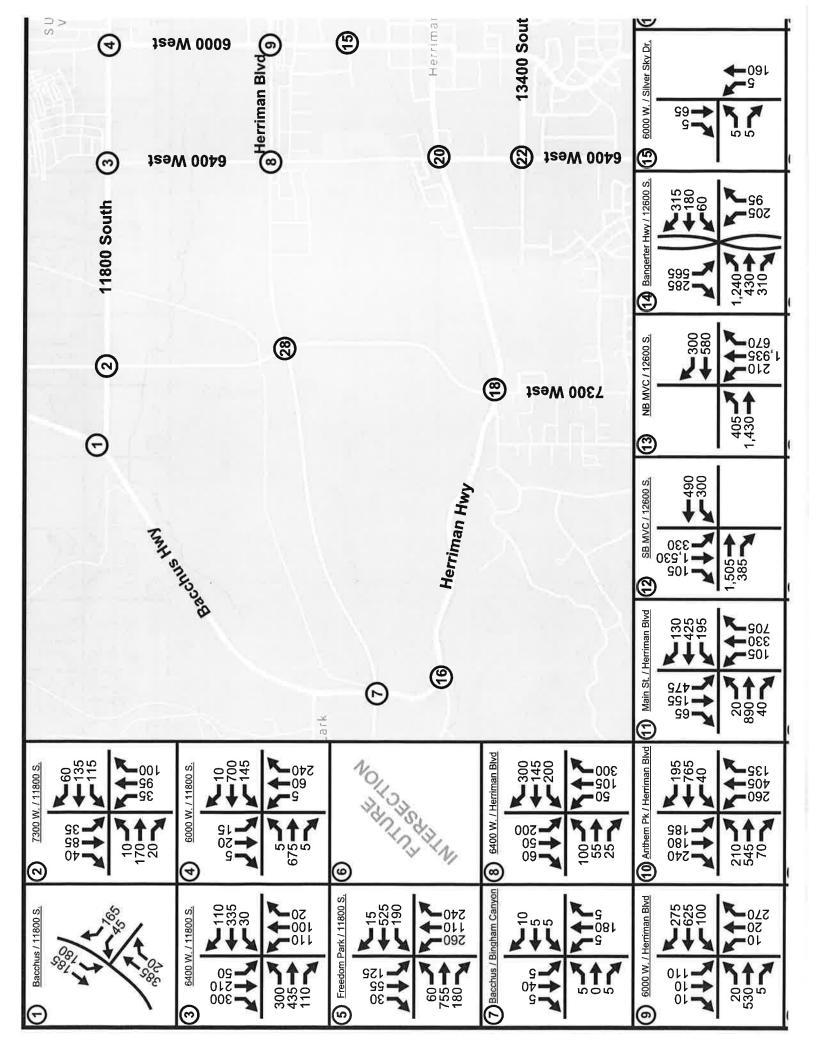
Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - Northbound Approach 515 feet (a.m. peak), 535 feet (p.m. peak)
- 6400 West / 11800 South
 - Northbound Approach 425 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Southbound Approach 780 feet (a.m. peak), >1,000 feet (p.m. peak)
- Freedom Park Drive / 11800 South
 - Southbound Approach 530 feet (p.m. peak)
- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach 500 feet (a.m. peak)
 - Westbound Approach 620 feet (a.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Northbound Approach 745 feet (a.m. peak)
 - Southbound Approach >1,000 feet (a.m. peak)
- Mountain View Corridor / 12600 South



- Northbound Approach 465 feet (a.m. peak), 430 feet (p.m. peak)
- Southbound Approach 560 feet (a.m. peak), >1,000 feet (p.m. peak)
- Eastbound Approach 630 feet (a.m. peak)
- Westbound Approach 405 feet (a.m. peak), >1,000 feet (p.m. peak)
- Bangerter Highway / 12600 South
 - Southbound Offramp 375 feet (a.m. peak)
 - Eastbound Approach 410 feet (a.m. peak)
- 6400 West / 13400 South
 - Northbound Approach 420 feet (p.m. peak)
 - Southbound Approach 375 feet (a.m. peak), 570 feet (p.m. peak)
 - Eastbound Approach 410 feet (a.m. peak)
- 5600 West / 13400 South
 - Northbound Approach 635 feet (a.m. peak), 400 feet (p.m. peak)
 - Eastbound Approach 995 feet (a.m. peak), 420 feet (p.m. peak)
- 5000 West / 13400 South
 - Southbound Approach >1,000 feet (a.m. peak)
 - Eastbound Approach >1,000 feet (a.m. peak)
- Mountain View Corridor / 13400 South
 - Northbound Approach >1,000 feet (a.m. and p.m. peak)
 - Southbound Approach 480 feet (a.m. peak)
 - Eastbound Approach 835 feet (a.m. peak), 445 feet (p.m. peak)
 - Westbound Approach 645 feet (a.m. peak), 595 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.



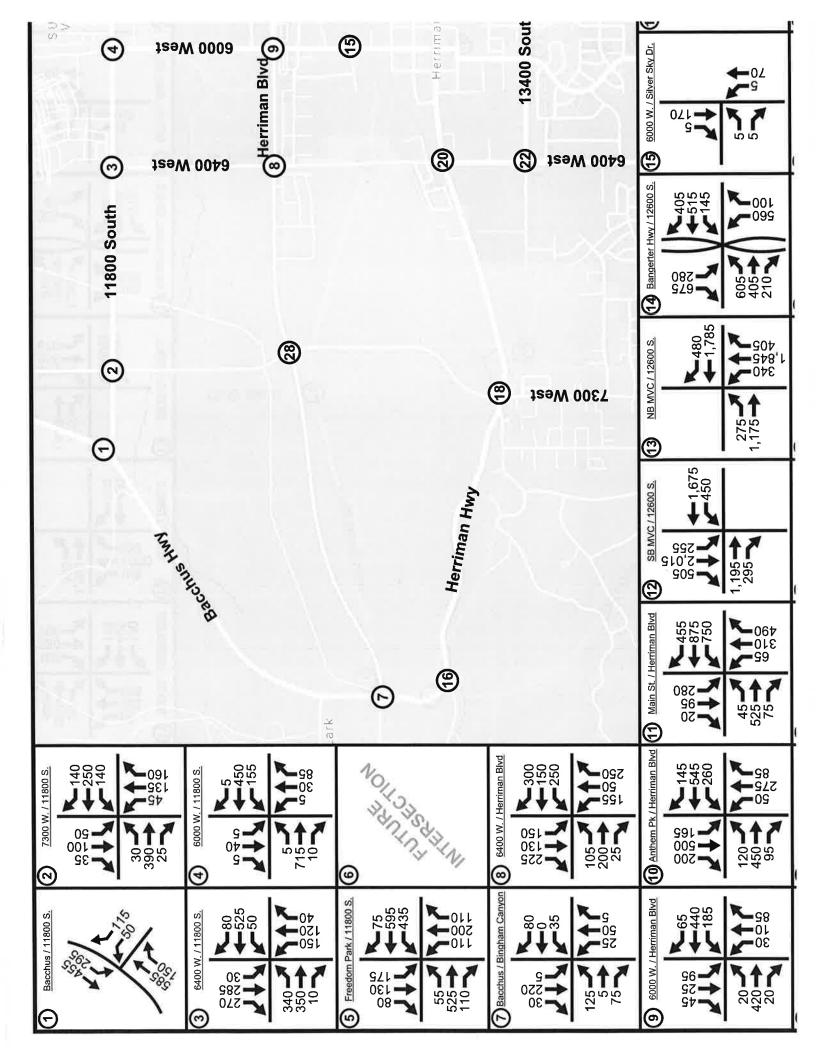




Table 11: Future (2032) Background Morning Peak Hour Level of Service

Intersection	itersection Worst				Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal	1, * :	(8)		35.5	D	
7300 West / 11800 South	NB/SB Stop	NB	10.7	В	.	B	P . .
6400 West / 11800 South	NB/SB Stop	SB	>75.0	F	3 8	16	B (15.7)
6000 West / 11800 South	Signal	9	<u> </u>		12.2	В	3
Freedom Park Drive / 11800 South	Signal	199	ie:		16.4	8	(*)
Bingham Canyon Mine / Bacchus Highway	EB/WB Stop	EB	2.9	Α	#\	-	**
7300 West / Herriman Boulevard	NB/SB Stop	SB	8.0	A	9))÷	8
6400 West / Herriman Boulevard	NB/SB Stop	NB	27.7	D	325		*
6000 West / Herriman Boulevard	Signal	360	æ	*	10.9	В	(#0
Anthem Park Boulevard / Herriman Boulevard	Signal	٠		983	39.1	D	*
Main Street / Herriman Boulevard / 12600 South	Signal	*		*	53.5	D	(*)
SB MVC / 12600 South	Signal	•	•	ĵ.	64.2	E	N/A
NB MVC / 12600 South	Signal		200	*	35.3	D	N/A
Bangerter Highway / 12600 South	Signal		340	2	34.6	С	14.5
Silver Sky Drive / 6000 West	EB Stop	EB	3.4	Α	36		3.
Butterfield Canyon Road / Herriman Highway / Bacchus <mark>Hig</mark> hway	EB Stop	ЕВ	4.1	А	4 :	22	æ
7300 West / Herriman Highway	NB/SB Stop	SB	10.7	В	8)		ž.
6400 West / Main Street	Signal	•		3	26.1	С	i.
5600 West / Main Street	Signal				13.7	В	
6400 West / 13400 South	Signal	- 160 L	(a)	2	18.6	В	- Sa
5600 West / 13400 South	Signal	•	•	•	53.9	D	÷
5000 West / 13400 South	Signal	144	·	-	>120.0	F	C (29.4)
SB MVC / 13400 South	Signal		545	ž.	107.1	F	N/A
NB MVC / 13400 South	Signal			Ţ	111.9	F	N/A

1. This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.

Source: Hales Engineering, November 2019



Table 12: Future (2032) Background Evening Peak Hour Level of Service

Intersection		Wor	st Approach	Overall Intersection			Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh)	LOS¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal			3	30.9	C	
7300 West / 11800 South	NB/SB Stop	NB	18.8	С	*	:E	\$
6400 West / 11800 South	NB/SB Stop	SB	>75.0	F	2 (0	C (25.9)
6000 West / 11800 South	Signal			- S	9.9	Α	- 56
Freedom Park Drive / 11800 South	Signal	ĝ.	Ē		28.2	С	7
Bingham Canyon Mine / Bacchus Highway	EB/WB Stop	EB	6.0	A	2		ě
7300 West / Herriman Boulevard	NB/SB Stop	NB	16.4	С	:#6	645	
6400 West / Herriman Boulevard	NB/SB Stop	NB	21.7	С	: *	:20	<u>s</u>
6000 West / Herriman Boulevard	Signal	÷	¥		9.0	A	:
Anthem Park Boulevard / Herriman Boulevard	Signal	æ	*	æ	19.0	В	9€
Main Street / Herriman Boulevard / 12600 South	Signal	9C	-	•	25.0	С	æ
SB MVC / 12600 South	Signal				99.9	F	N/A
NB MVC / 12600 South	Signal)	-	(*)	112.8	F	N/A
Bangerter Highway / 12600 South	Signal	- 21	8	21	26.8	С	*
Silver Sky Drive / 6000 West	EB Stop	EB	3.4	Α			
Butterfield Canyon Road / Herriman Highway /	EB Stop	EB	4.2	A	Ser.	4	840
Pacchus Highway 7300 West / Herriman Highway	NB/SB Stop	SB	13.7	В	*	ě	·
6400 West / Main Street	Signal	32 7	2	7 <u>5</u> (i	28.0	С	120
5600 West / Main Street	Signal	##/s		2/	15.1	В	120
6400 West / 13400 South	Signal	:€;		<u>(€</u>);	23.7	С	
5600 West / 13400 South	Signal	38		548	39.0	D	
5000 West / 13400 South	Signal	<u></u>	÷		23.8	С	•
SB MVC / 13400 South	Signal	*:		\$ 9 .5	36.4	D	285
NB MVC / 13400 South	Signal	(4)	-	360	84.1	F	N/A

¹ This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

^{3.} SB = Southbound approach, etc

Source: Hales Engineering, November 2019



F. Mitigation Measures

A northbound right-turn pocket is recommended on the northbound approach to the Bacchus Highway / 11800 South intersection is recommended to mitigate the queues that are anticipated on that approach.

It is anticipated that both morning and evening peak hour traffic volumes in 2032 at the 6400 West / 11800 South intersection will warrant the installation of a traffic signal (based on Utah MUTCD 2009 Chapter 4C Warrant 3). Therefore, a traffic signal is recommended at this intersection. It is also recommended that permissive/protected left-turn phasing be installed on the east- and westbound approaches, and that right-turn pockets be constructed on the north- and southbound approaches.

Hales Engineering analyzed a mitigated scenario which assumed that these recommended mitigation measures had been implemented. The mitigated scenario also assumed that additional improvements had been made at the Mountain View Corridor / 12600 South and Mountain View Corridor / 13400 South intersections. These improvements include channelized east- and westbound right-turn lanes at both intersections, the conversion of a northbound through lane at 12600 South to a shared through/left-turn lane and extending the three eastbound lanes on 13400 south to 5000 West.

With the recommended mitigation measures, the 6400 West / 1800 South intersection is anticipated to improve to an acceptable level of service in both the morning and evening peak hours. The poor levels of service and excessive queuing at the Mountain View Corridor / 12600 South and Mountain View Corridor / 13400 South intersections is anticipated to persist. Along with the poor levels of service, the excessive queueing at the Mountain View Corridor / 13400 South intersection is adversely impacting other intersections on 13400 South.

Projected traffic conditions at the Mountain View Corridor / 12600 South and Mountain View Corridor / 13400 South intersections are anticipated to be such that the mitigation measures required to attain acceptable levels of service exceed that of this traffic impact study. Hales Engineering acknowledges that capacity enhancements will be needed at these locations, but those enhancements will need to be developed at a system level by UDOT or other entities. Therefore, the Mountain View Corridor / 12600 South, Bangerter Highway / 12600 South, and Mountain View Corridor / 13400 South intersections will be omitted from further analyses.

An additional mitigated scenario was analyzed which assumed that the queueing at the Mountain View Corridor intersections had been mitigated. With this assumption the poor level of service during the morning peak hour at the 5000 West / 13400 South intersection is anticipated to improve to LOS C.



VIII. FUTURE (2032) PLUS PROJECT CONDITIONS

A. Purpose

The purpose of the future (2032) plus project analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions plus the net trips generated by the proposed development. This scenario provides valuable insight into the potential impacts of the proposed project on future background traffic conditions.

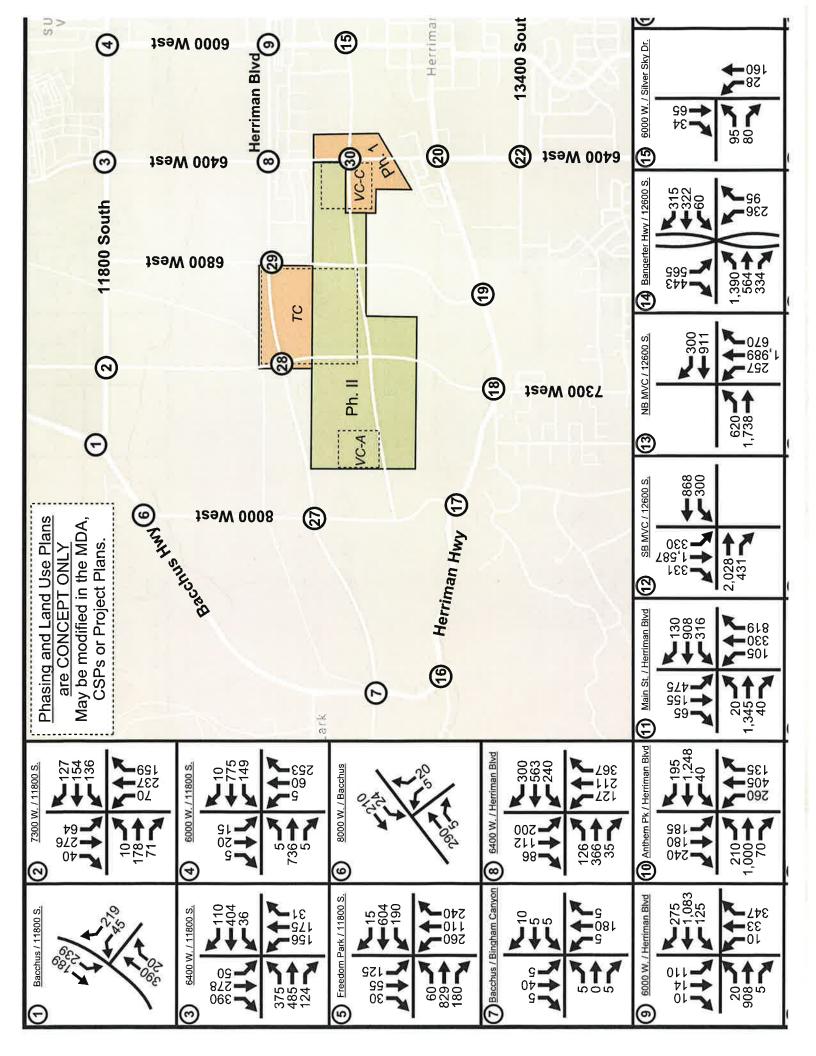
B. Traffic Volumes

Hales Engineering added the Phase II project trips discussed in Chapter V to the future (2032) background traffic volumes to predict turning movement volumes for future (2032) plus project conditions. Additional turning movement volumes were added manually to new project roadways as well to match better with the volumes provided by Horrocks in the build travel demand models. Future (2032) plus project evening peak hour turning movement volumes are shown in Figure 22 and Figure 23.

C. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at LOS E or LOS F in future (2032) plus project conditions as shown in Table 13 and Table 14:

- 7300 West / 11800 South (Morning and Evening Peak)
- Anthem Park Boulevard / Herriman Boulevard (Morning Peak)
- Main Street / Herriman Boulevard (Morning and Evening Peak)
- 6800 West / Herriman Highway (Evening Peak)
- 6400 West / Main Street (Evening Peak)
- 6400 West / 13400 South (Morning Peak)
- 5600 West / 13400 South (Evening Peak)
- 5000 West / 13400 South (Evening Peak)
- 7300 West / Herriman Boulevard (Morning and Evening Peak)
- 6800 West / Herriman Boulevard (Morning and Evening Peak)



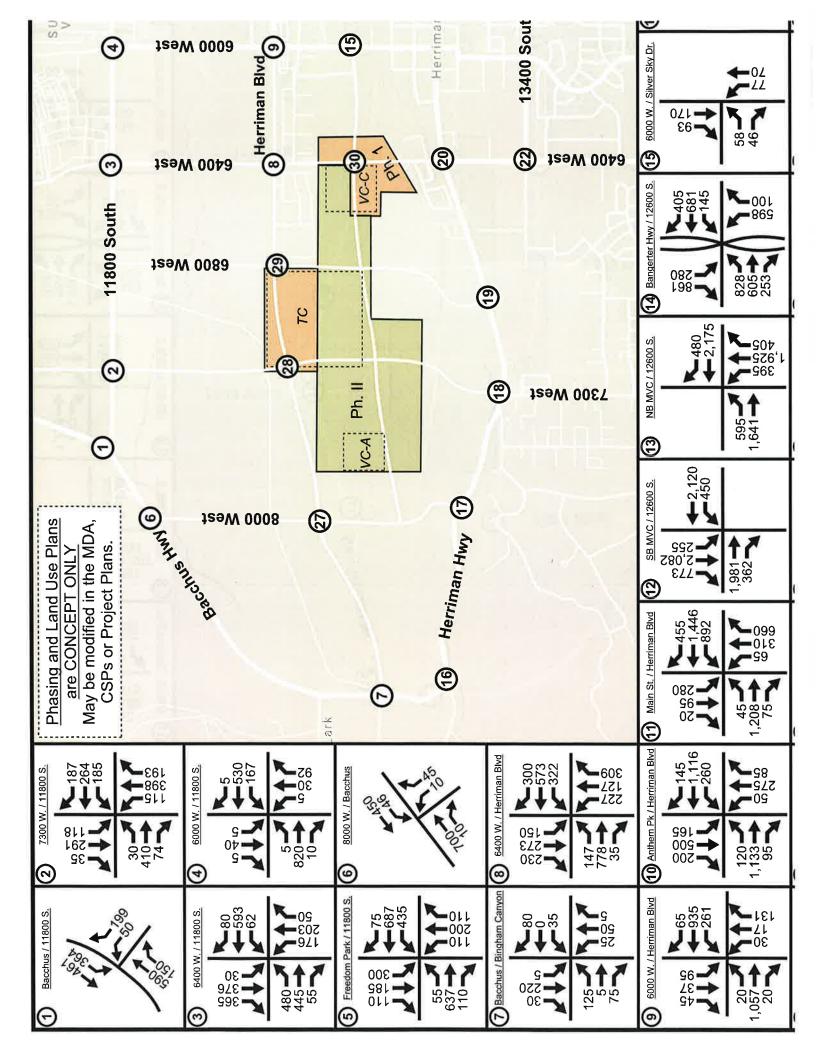




Table 13: Future (2032) Plus Project Morning Peak Hour Level of Service

Intersection		Wor	st Approach	- 1	Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal	19		•	34.5	С	9
7300 West / 11800 South	NB Stop	NB	60.6	F		0.60	B (11.3)
6400 West / 11800 South	Signal	ű k s	525	S.	22.4	С	ž.
6000 West / 11800 South	Signal	(場)	:	5.	13.0	В	•
Freedom Park Drive / 11800 South	Signal	::•□	: + 0	*	22.7	С	-
8000 West / Bacchus Highway	NB Stop	NB	4.0	Α	₹	· •	22
Bingham Canyon Mine / Bacchus Highway	EB Stop	EB	3.4	A	ž.	3 3 5	ş
6400 West / Herriman Boulevard	Signal	62	€	<u>ş</u>	29.5	С	8
6000 West / Herriman Boulevard	Signal	\$:	Ge:	2	16.3	В	ŝ
Anthem Park Boulevard / Herriman Boulevard	Signal	3 = 3	; . :	٠	65.1	E	D (43.9)
Main Street / Herriman Boulevard / 12600 South	Signal		2	Ę	102.3	F	D (53.5)
Silver Sky Drive / 6000 West	EB Stop	EB	5.4	Α	_ s	25	
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	ЕВ	3.7	Α	2	6	÷
8000 West / Herriman Highway	SB Stop	SB	5.1	Α	·	:6:	-
7300 West / Herriman Highway	NB/SB Stop	NB	23.4	С	×	*	*
6800 West / Herriman Highway	SB Stop	SB	11.9	В	•	.*	
6400 West / Main Street	Signal	25	-	ž	35.0	С	-
5600 West / Main Street	Signal	18:	15	*	16.6	В	¥
6400 West / 13400 South	Signal	189	-	*	62.9	Е	D (36.5)
5600 West / 13400 South	Signal		æ	- 2	38.1	D	
5000 West / 13400 South	Signal	:#:	le:		45.4	D	¥
8000 West / Herriman Boulevard	NB/SB Stop	SB	5.5	А	5		
7300 West / Herriman Boulevard	NB/SB Stop	NB	72.7	F	•	929	B (15.2)
6800 West / Herriman Boulevard	NB/SB Stop	SB	>75.0	F		•	B (14.5)
Silver Sky Drive / 6400 West	EB/WB Stop	ЕВ	33.2	D	ĝ		<u> </u>

Source: Hales Engineering, November 2019



Table 14: Future (2032) Plus Project Evening Peak Hour Level of Service

Intersection		Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	1, 1, 3	Aver. Delay	2002	Aver. Delay	Loc2	LOC (Dalaw)
Description	Control	Approach ^{1,3}	(Sec/Veh)	LOS	(Sec/Veh)2	LOS	LOS (Delay)
Bacchus Highway / 11800 South	Signal			3.00	29.4	С	•
7300 West / 11800 South	NB Stop	NB	>75.0	F			B (17.0)
6400 West / 11800 South	Signal				48.1	D	
6000 West / 11800 South	Signal	5.	a l	:#:	11.7	В	
Freedom Park Drive / 11800 South	Signal				30.3	C	
8000 West / Bacchus Highway	NB Stop	NB	8.5	Α	-	3€ 3	
Bingham Canyon Mine /	EB Stop	EB	5.5	A			-
Bacchus Highway	ев этор	EB	5.5	A		363	
6400 West / Herriman Boulevard	Signal		<u> </u>	-	41.5	D	
6000 West /	Signal				15.3	В	
Herriman Boulevard	Signal				13.5	В	
Anthem Park Boulevard / Herriman	Signal	·•/			40.0	D	
Boulevard	Signal			U.F.1	40.0		
Main Street / Herriman Boulevard /	Signal			74	56.7	Е	C (34.1)
12600 South	Signal				30.7	-	C (34.1)
Silver Sky Drive / 6000 West	EB Stop	EB	6.1	Α	¥	*	<u> </u>
Butterfield Canyon Road /							
Herriman Highway /	EB Stop	EB	4.1	A	=	(*)	¥
Bacchus Highway							
8000 West / Herriman Highway	SB Stop	SB	6.5	Α	*		
7300 West / Herriman Highway	NB/SB	SB	30.2	D	D -		
7300 West/ Herriman Highway	Stop	36	50.2			578	
6800 West / Herriman Highway	SB Stop	SB	51.2	F			B (14.7)
6400 West / Main Street	Signal	? ≠ (t	;•0	2.00	>120.0	F	D (54.4)
5600 West / Main Street	Signal	9	*	14	43.8	D	
6400 West / 13400 South	Signal	. 	:#S	: ::::	49.8	D	
5600 West / 13400 South	Signal	: • :	745	•	>120.0	F	D (53.4)
5000 West / 13400 South	Signal	-	-	•	60.7	Ē	C (24.0)
8000 West / Herriman Boulevard	NB/SB	SB	6.1	А		-	42
BOOD West/ Herrifian Boulevard	Stop	30	0.1				
7300 West / Herriman Boulevard	NB/SB	SB	>75.0	F			C (28.7)
	Stop	36	~75.0				C (25.7)
6800 West / Herriman Boulevard	NB/SB	SB	>75.0	F	_		B (18.4)
Trest, Hellinian boulevalu	Stop			-35	D (10.4)		
Silver Sky Drive / 6400 West	EB/WB	EB	18.1	С	2	2	
Sirter only office / vivo treat	Stop		20:1				

[.] This represents the worst approach LOS and delay (seconds I vehicle) and is only reported for non-all-way stop unsignalized intersections. 2. This represents the overall intersection LOS and delay (seconds I vehicle) and is reported for all-way stop and signal-controlled intersections 3.55 = Southbound approach, etc.

Source: Hales Engineering, November 2019



D. Queuing Analysis

Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - Northbound Approach 490 feet (p.m. peak)
- 7300 West / 11800 South
 - Northbound Approach 635 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Southbound Approach 415 feet (a.m. peak), >1,000 feet (p.m. peak)
- 6400 West / 11800 South
 - Northbound Approach 760 feet (p.m. peak)
 - Southbound Approach 400 feet (p.m. peak)
 - Eastbound Approach 625 feet (p.m. peak)
 - Westbound Approach 365 feet (p.m. peak)
- Freedom Park Drive / 11800 South
 - Southbound Approach 400 feet (p.m. peak)
- 6400 West / Herriman Boulevard
 - o Northbound Approach 435 feet (a.m. peak), 635 feet (p.m. peak)
 - Southbound Approach 505 feet (p.m. peak)
 - Westbound Approach 400 feet (p.m. peak)
- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach 430 feet (a.m. peak)
 - Southbound Approach 400 feet (p.m. peak)
 - Eastbound Approach 680 feet (a.m. peak), 570 feet (p.m. peak)
 - Westbound Approach ->1,000 feet (a.m. peak), 470 feet (p.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Northbound Approach 875 feet (a.m. peak), 460 feet (p.m. peak)
 - Southbound Approach ->1,000 feet (a.m. peak), 575 feet (p.m. peak)
 - Eastbound Approach ->1,000 feet (a.m. peak), 605 feet (p.m. peak)
 - Westbound Approach 470 feet (p.m. peak)
- 6400 West / Main Street
 - Northbound Approach 820 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Southbound Approach 430 feet (a.m. peak), 730 feet (p.m. peak)
 - Eastbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach 757 feet (p.m. peak)
- 5600 West / Main Street
 - Southbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach 720 feet (p.m. peak)



- 6400 West / 13400 South
 - Northbound Approach >1,000 feet (a.m. peak)
 - Southbound Approach 955 feet (a.m. peak), >1,000 feet (p.m. peak)
- 5600 West / 13400 South
 - Northbound Approach 425 feet (a.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
 - o Eastbound Approach 490 feet (a.m. peak), 595 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 5000 West / 13400 South
 - Southbound Approach >1,000 feet (a.m. and p.m. peak)
 - Eastbound Approach 465 feet (a.m. peak), 390 feet (p.m. peak)
 - Westbound Approach 930 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.

E. Mitigation Measures

It is anticipated that a traffic signal will be warranted at the 6400 West / Herriman Boulevard intersection. It is recommended that a traffic signal be installed when warranted. In order to mitigate the anticipated queueing at the intersection, it is recommended that right-turn lanes be added on all approaches and that permissive-protected phasing be implemented on the eastbound and westbound approaches.

It is anticipated that a traffic signal will be warranted at the 7300 West / 11800 South intersection. It is recommended that a traffic signal be installed with turn pockets when warranted.

At the Anthem Park Boulevard / Herriman Boulevard intersection, it is recommended that the cycle length be increased to 150 seconds and that the northbound right-turn lane be converted to a shared through-right lane.

At the Main Street / Herriman Boulevard intersection, it is recommended that the cycle length be increased to 150 seconds, that a second northbound through lane be added, and that the eastbound right-turn lane be converted to a shared through-right lane.

It is anticipated that a traffic signal will be warranted at the 6800 West / Herriman Highway intersection. It is recommended that a traffic signal be installed with turn pockets when warranted.

At the 6400 West / Main Street intersection, it is recommended that right-turn lanes be added on all approaches, that permissive-protected phasing be implemented on the north- and southbound approaches, and that a right-turn overlap phase be implemented on the eastbound approach.



At the 6400 West / 13400 South intersection, it is recommended that the cycle length be increased to 120 seconds, that dual left-turns be installed on the south- and westbound approaches, and that a right-turn overlap phase be implemented on the westbound approach.

It is recommended that 13400 South be widened to seven lanes between 5000 West and 5600 West and to five lanes between 5600 West and 6400 West to provide needed capacity at the study intersections.

At the 5600 West / 13400 South intersection, it is recommended that right-turn lanes be added on all approaches.

At the 5000 West / 13400 South intersection, it is recommended that a right-turn lane be added on the westbound approach.

It is anticipated that a traffic signal will be warranted at the 7300 West / Herriman Boulevard intersection. It is recommended that a traffic signal be installed with turn pockets when warranted.

It is anticipated that a traffic signal will be warranted at the 6800 West / Herriman Boulevard intersection. It is recommended that a traffic signal be installed with turn pockets when warranted and that permissive-protected phasing be implemented on the eastbound approach.

In order to mitigate queueing at the 6400 West / 11800 South intersection, it is recommended that right-turn lanes be added on the eastbound and westbound approaches and that permissive-protected phasing be implemented on the northbound approach.

In order to mitigate queueing at the 7300 West / Herriman Highway intersection, it is recommended that a right-turn lane be added on the westbound approach.

Hales Engineering completed a mitigated scenario with the proposed improvements. Based on the mitigated scenario, is anticipated that the proposed improvements will improve the LOS at all study intersections to an acceptable level of service.



FUTURE (2037) BACKGROUND CONDITIONS

A. Purpose

The purpose of the future (2037) background analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions. Through this analysis, future background traffic operational deficiencies can be identified, and potential mitigation measures recommended.

B. Roadway Network

It was assumed that all previously recommended background mitigation measures had been implemented prior to 2037. It was also assumed that all traffic signals had been coordinated to optimize traffic flow along the 11800 South, Herriman Boulevard, and 13400 South corridors.

C. Traffic Volumes

Hales Engineering obtained future (2037) forecasted volumes from a modified version of the WFRC / MAG TDM. This version of the WFRC/MAG TDM was tailored specifically for this project by Horrocks Engineers (and reviewed by Salt Lake County) to forecast future average weekday daily traffic (AWDT) volumes within the study area. Peak period turning movement counts were estimated using National Cooperative Highway Research Program (NCHRP) 255 methodologies which utilize existing peak period turn volumes and future AWDT volumes to project the future turn volumes at the major intersections. Future (2037) morning and evening peak hour turning movement volumes are shown in Figure 24 and Figure 25.

D. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at LOS E or LOS F in future (2037) background conditions as shown in Table 15 and Table 16:

- 7300 West / 11800 South (Morning and Evening Peak)
- 7300 West / Herriman Boulevard (Evening Peak)
- 6400 West / Herriman Boulevard (Morning and Evening Peak)
- Anthem Park Boulevard / Herriman Boulevard (Morning Peak)
- 7300 West / Herriman Highway (Morning and Evening Peak)
- 5600 West / 13400 South (Evening Peak)

These results serve as a baseline condition for the impact analysis of the proposed development for future (2037) conditions.



E. Queuing Analysis

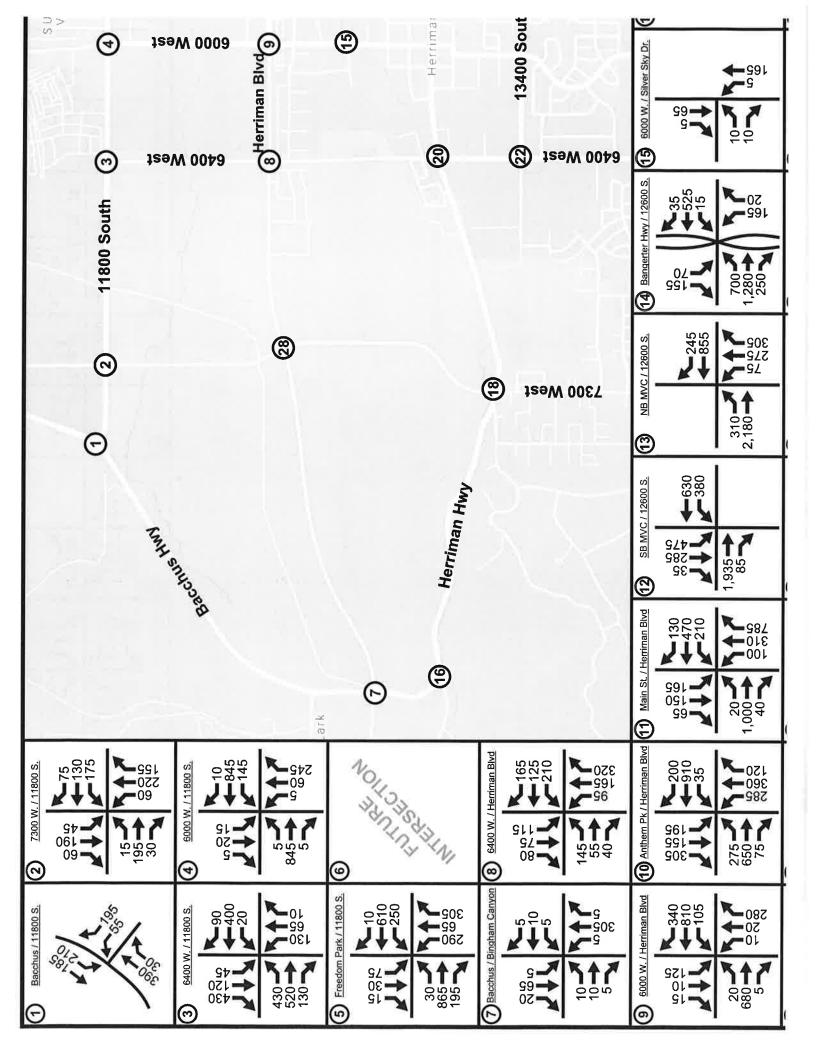
Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - Northbound Approach 585 feet (a.m. peak), 525 feet (p.m. peak)
- 7300 West / 11800 South
 - Northbound Approach 390 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
- 6400 West / 11800 South
 - Northbound Approach 750 feet (p.m. peak)
 - Eastbound Approach 355 feet (p.m. peak)
- 7300 West / Herriman Boulevard
 - Northbound Approach 815 feet (p.m. peak)
 - Eastbound Approach 690 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 6400 West / Herriman Boulevard
 - Northbound Approach 715 feet (a.m. peak, >1,000 feet (p.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach >1,000 feet (a.m. peak)
 - Southbound Approach 560 feet (a.m. peak), 515 feet (p.m. peak)
 - Eastbound Approach 515 feet (a.m. peak)
 - Westbound Approach 700 feet (a.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Southbound Approach 380 feet (a.m. peak)
 - Westbound Approach 795 feet (p.m. peak)
- 7300 West / Herriman Highway
 - Northbound Approach >1,000 feet (a.m. and p.m. peak)
 - Southbound Approach 410 feet (p.m. peak)
- 6400 West / Main Street
 - Northbound Approach 355 feet (a.m. peak)
 - Southbound Approach 380 feet (a.m. peak), 350 feet (p.m. peak)
- 5600 West / Main Street
 - Northbound Approach 550 feet (a.m. peak), 350 feet (p.m. peak)
 - Southbound Approach 515 feet (p.m. peak)
- 6400 West / 13400 South
 - Northbound Approach 530 feet (a.m. peak)
 - Southbound Approach 615 feet (p.m. peak)



- 5600 West / 13400 South
 - o Northbound Approach 495 feet (a.m. peak), 380 feet (p.m. peak)
 - Eastbound Approach 645 feet (a.m. peak), 805 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 5000 West / 13400 South
 - Southbound Approach 990 feet (a.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.



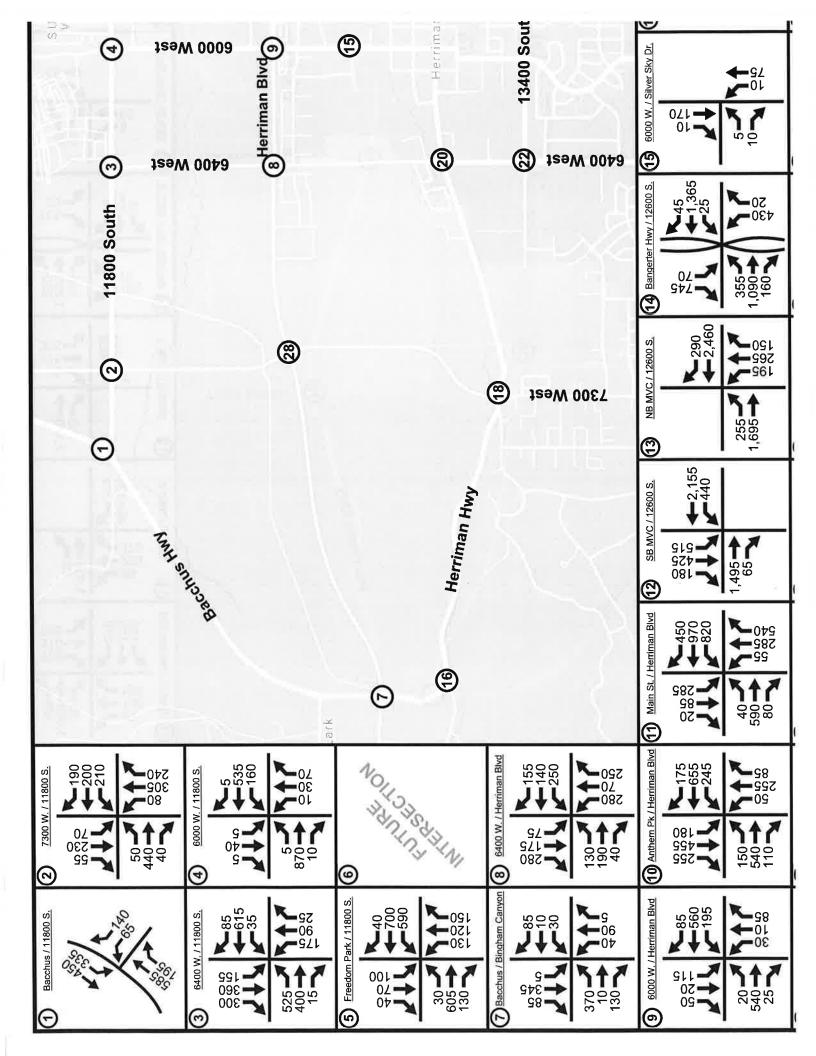




Table 15: Future (2037) Background Morning Peak Hour Level of Service

Intersection	200	Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal		•	-	36.9	С	
7300 West / 11800 South	NB/SB Stop	NB	36.5	E			B (17.1)
6400 West / 11800 South	Signal	•			16.9	В	(•)
6000 West / 11800 South	Signal		3	3	13.3	В	•
Freedom Park Drive / 11800 South	Signal		*	*	21.6	С	ne:
Bingham Canyon Mine /	EB/WB	ED.	6.5	А	2		
Bacchus Highway	Stop	EB	0.5	А		-	
7300 West / Herriman Boulevard	NB/SB Stop	SB	10.2	В	·	×	
6400 West / Herriman Boulevard	NB/SB Stop	NB	52.9	F		÷	B (15.5)
6000 West / Herriman Boulevard	Signal	•		4	12.2	В	
Anthem Park Boulevard / Herriman Boulevard	Signal		×	¥	57.8	E	D (46.7)
Main Street / Herriman Boulevard / 12600 South	Signal		4	-	22.2	С	790
SB MVC / 12600 South	Signal	100	4	¥	-	¥	Viet.
NB MVC / 12600 South	Signal	_(#)			E	15.	
Bangerter Highway / 12600 South	Signal	·	4	÷	E:	i i	16
Silver Sky Drive / 6000 West	EB Stop	EB	4.0	A	5	7.5	
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	EB	3.9	Α	5.	P.	(Jes
7300 West / Herriman Highway	NB/SB Stop	NB	>120.0	F	ě	16	B (18.6)
6400 West / Main Street	Signal		•	5	27.1	С	2.5
5600 West / Main Street	Signal	(2)		-	23.4	С	226
6400 West / 13400 South	Signal				23.8	С	38
5600 West / 13400 South	Signal			-	37.1	D	2.0
5000 West / 13400 South	Signal		,	÷	38.2	D	(\ = 2
SB MVC / 13400 South	Signal			=	<u> </u>	:4:	\@E
NB MVC / 13400 South	Signal		•	-),5	

^{1.} This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

SB = Southbound approach, etc.

Source: Hales Engineering, November 2019



Table 16: Future (2037) Background Evening Peak Hour Level of Service

Intersection	V 100 T-	Wor	st Approach	100	Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOSZ	LOS (Delay)
Bacchus Highway / 11800 South	Signal	12	4	2	28.9	С	201
7300 West / 11800 South	NB/SB Stop	NB	>120.0	F	*	20	C (25.6)
6400 West / 11800 South	Signal	18 9 2			48.5	D	
6000 West / 11800 South	Signal	7 2 :	a	ä	11.9	В	**
Freedom Park Drive / 11800 South	Signal	:::::::::::::::::::::::::::::::::::::::		- 5	21.1	С	
Bingham Canyon Mine / Bacchus Highway	EB/WB Stop	ЕВ	14.4	В	*	(• €)	•:
7300 West / Herriman Boulevard	NB/SB Stop	NB	>75.0	F	2 3	:• 0	D (40.1)
6400 West / Herriman Boulevard	NB/SB Stop	SB	> <mark>75.</mark> 0	F	98	*	B (17.2)
6000 West / Herriman Boulevard	Signal	(vez			11.1	В	.5)
Anthem Park Boulevard / Herriman Boulevard	Signal	Œ	Ē	ě	27.5	С	
Main Street / Herriman Boulevard / 12600 South	Signal	優	Ē		50.1	D	1.5
SB MVC / 12600 South	Signal)≤	=	2	U t s		
NB MVC / 12600 South	Signal	160	*	- =	9 + 0	140	3#0
Bangerter Highway / 12600 South	Signal	•	ŝ	ŝ		•	
Silver Sky Drive / 6000 West	EB Stop	EB	3.4	Α	190	200)*:
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	EB	5.0	A	Des	(#2	*
7300 West / Herriman Highway	NB/SB Stop	NB	>120.0	F	8	•	C (32.6)
6400 West / Main Street	Signal		*	÷	28.8	С	
5600 West / Main Street	Signal	4	¥:		22.6	С	\$ 6 0
6400 West / 13400 South	Signal	3		÷	27.4	С	
5600 West / 13400 South	Signal	#	90		>120.0	F	D (42.7)
5000 West / 13400 South	Signal	- 8		•	54.5	D	•
SB MVC / 13400 South	Signal	#			(*c		
NB MVC / 13400 South	Signal		ä.		144	•	

1. This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.



F. Mitigation Measures

It is anticipated that traffic signals will be warranted at the following intersections based on projected 2037 traffic volumes:

- 7300 West / 11800 South
- 7300 West / Herriman Boulevard
- 6400 West / Herriman Boulevard
- 7300 West / Herriman Highway

It is recommended that traffic signals be installed at these intersections when appropriate warrants are met. In addition to a traffic signal, it is recommended that right-turn lanes be constructed on the north- and southbound approaches to the 6400 West / Herriman Boulevard intersection.

It is recommended that dual left-turn lanes be constructed on the eastbound approach to the 6400 West / 11800 South intersection, and that permissive/protected left-turn phasing be installed on the north- and southbound approaches. Dual left-turn lanes are also recommended on the eastbound approach to the Anthem Park Boulevard / Herriman Boulevard intersection.

It is recommended that separate left- and right-turn lanes be constructed on the northbound approach to the 7300 West / Herriman Highway intersection. According to the WFRC RTP, 7300 West is planned to be expanded to a five-lane cross section south of Herriman Highway as a Phase 2 (2031-2040) project. This planned improvement would coincide with the recommended improvement at the intersection.

It is anticipated that additional capacity will be needed at the 5600 West / 13400 South intersection. It is recommended that the left- and right-turn lanes on all approaches to the 5600 West / 13400 South intersection be extended to accommodate the anticipated queueing and that separate right-turn lanes be added to the south- and eastbound approaches. It is also recommended that the five-lane cross section on 13400 South be extended to the west of 5600 West and that an additional through lane be added to the north- and southbound approaches.

Hales Engineering analyzed a mitigated scenario which assumed that these recommended mitigation measures had been implemented. Based on this analysis the recommended mitigation measures are anticipated to result in acceptable levels of service throughout the study area.

No additional mitigation measures are recommended.



IX. FUTURE (2037) PLUS PROJECT CONDITIONS

A. Purpose

The purpose of the future (2037) plus project analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions plus the net trips generated by the proposed development. This scenario provides valuable insight into the potential impacts of the proposed project on future background traffic conditions.

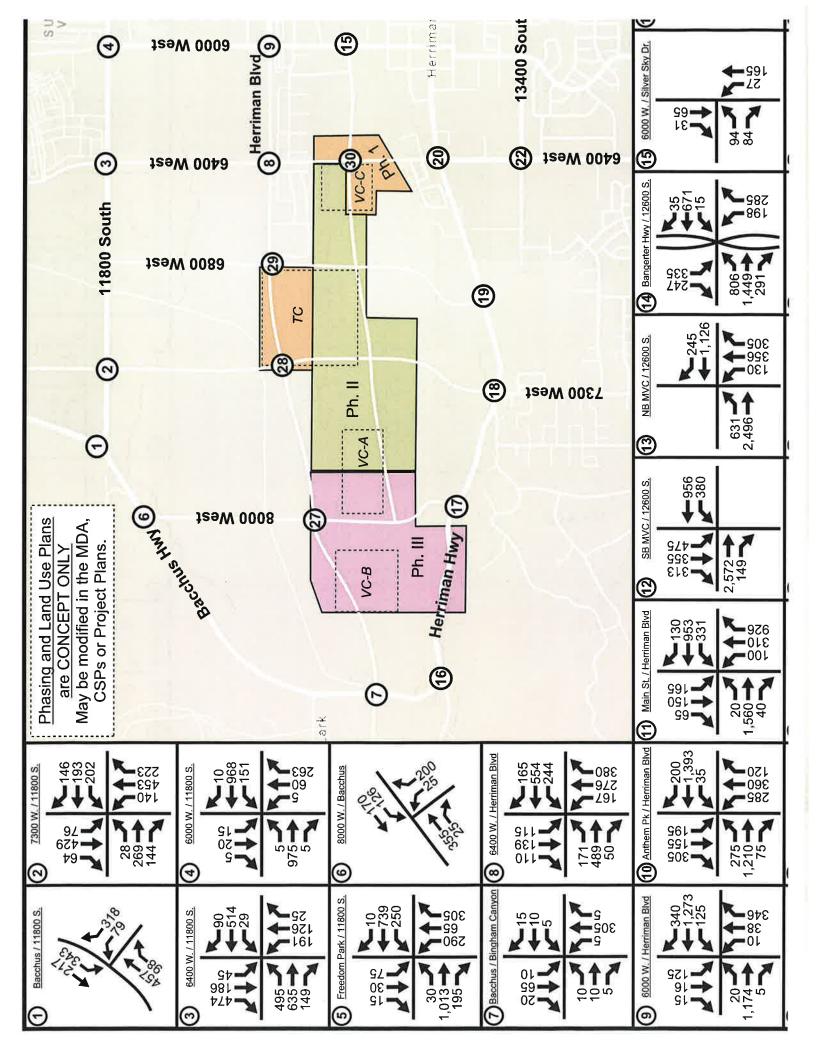
B. Traffic Volumes

Hales Engineering added the Phase III project trips discussed in Chapter V to the future (2037) background traffic volumes to predict turning movement volumes for future (2037) plus project conditions. Additional turning movement volumes were added manually to new project roadways as well to match better with the volumes provided by Horrocks in the build travel demand models. Future (2037) plus project evening peak hour turning movement volumes are shown in Figure 26 and Figure 27.

C. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at acceptable levels of service during the morning peak hour in future (2037) plus project conditions as shown in Table 17. The following intersections are anticipated to operate at LOS E or LOS F during the evening peak hour as shown in Table 18:

- 7300 West / 11800 South
- 6400 West / 11800 South
- 7300 West / Herriman Highway
- 6400 West / Main Street
- 6400 West / 13400 South
- 7300 West / Herriman Boulevard



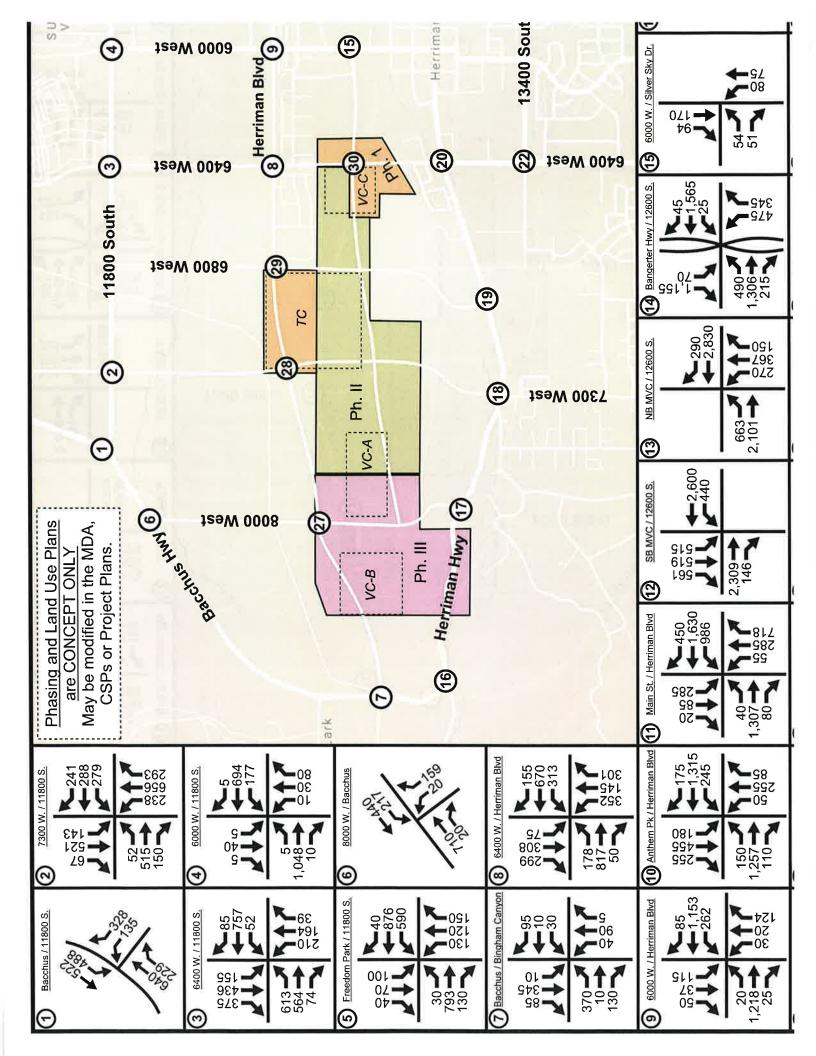




Table 17: Future (2037) Plus Project Morning Peak Hour Level of Service

Intersection		Wors	t Approach	, j	Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay	LOS1	Aver. Delay	LOS ²	LOS (Delay)
Description	Control	Approach	(Sec/Veh)	LUS	(Sec/Veh) ²	LOS	LOS (DElay)
Bacchus Highway / 11800 South	Signal	9	¥	=	32.0	С	,~
7300 West / 11800 South	Signal	*	•		30.0	С	
6400 West / 11800 South	Signal	-	£1	16	24.3	С	19
6000 West / 11800 South	Signal		•	×	15.5	В	/ *
Freedom Park Drive / 11800 South	Signal	3		•	23.4	С	
8000 West / Bacchus Highway	NB Stop	NB	8.8	Α	-	-	
Bingham Canyon Mine /	EB/WB	EB	5.0	А		1700.7	
Bacchus Highway	Stop	СВ	5.0	A	98	12	
6400 West / Herriman Boulevard	Signal	ě	•	¥	21.4	С	3
6000 West /	Cianal				21.6	С	
Herriman Boulevard	Signal				21.0	C	
Anthem Park Boulevard / Herriman	Cignal				46.0	D	
Boulevard	Signal	ž.			40.0	U	
Main Street / Herriman Boulevard /	Cignal				27.6	D	
12 <mark>600</mark> South	Signal			1.0	37.6	U	
Silver Sky Drive / 6000 West	EB Stop	EB	5.5	Α	35	-	
Butterfield Canyon Road /							
Herriman Highway /	EB Stop	EB	3.1	A	5 9 8		9346
Bacchus Highway							
8000 West / Herriman Highway	SB Stop	SB	8.3	Α	·=:	: ¥:	V#1
7300 West / Herriman Highway	Signal	-	7.54	0.56	49.0	D	
6800 West / Herriman Highway	Signal	-	(1)等	7.64	22.6	С	
6400 West / Main Street	Signal	•	1(8)	(19)	33.0	С	
5600 West / Main Street	Signal	-	S.₩	92	22.1	С	16#9
6400 West / 13400 South	Signal		1.50		26.2	С	
5600 West / 13400 South	Signal	-	9.5	3¥	27.6	С	744
5000 West / 13400 South	Signal		ue.	1,50	29.7	С	
8000 West / Herriman Boulevard	NB/SB	CD	7.5	Α			
8000 West / Herriman Boulevard	Stop	SB	7.5	A			S#1
7300 West / Herriman Boulevard	Signal				32.7	С	<u> </u>
6800 West / Herriman Boulevard	Signal	ė		Spi	30.0	С	28
Silver Sky Drive / 6400 West	EB/WB Stop	EB	20.5	С			8 =

^{1.} This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.

Source: Hales Engineering, November 2019



Table 18: Future (2037) Plus Project Evening Peak Hour Level of Service

Intersection		Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS²	LOS (Delay)
Bacchus Highway / 11800 South	Signal		195	3.5	29.8	С	6.5
7300 West / 11800 South	Signal		@	Œ	112.1	F	C (31.3)
6400 West / 11800 South	Signal	-	382	15 € 1	59.2	Е	D (52.0)
6000 West / 11800 South	Signal	Ē			11.9	В	<u> </u>
Freedom Park Drive / 11800 South	Signal	-)(#)		22.5	С	200
8000 West / Bacchus Highway	NB Stop	NB	15.0	В	<u>.</u>	-	
Bingham Canyon Mine /	EB/WB	EB	11.2	В			
Bacchus Highway	Stop	EB	11.2	ь		:=:	X
6400 West / Herriman Boulevard	Signal	펄)è:	76:	52.8	D	R#1
6000 West / Herriman Boulevard	Signal	Ä	æ	4	18.6	В	/æ
Anthem Park Boulevard / Herriman Boulevard	Signal	ä	÷	-	48.0	D	12
Main Street / Herriman Boulevard / 12600 South	Signal		· ·	19	32.7	С	-
Silver Sky Drive / 6000 West	EB Stop	EB	7.0	Α	N#S	c e	1,5
Butterfield Canyon Road /							
Herriman Highway /	EB Stop	EB	4.3	Α	7. 	16	150
Bacchus Highway							
8000 West / Herriman Highway	SB Stop	SB	11.6	В	<u> </u>	856	
7300 West / Herriman Highway	Signal	==	2	¥	66.0	E	D (49.2)
6800 West / Herriman Highway	Signal	*			52.6	D	
6400 West / Main Street	Signal	a	=	3	90.1	F	D (41.2)
5600 West / Main Street	Signal	-	•	*	30.1	С	-
6400 West / 13400 South	Signal	81		¥.	78.5	Е	C (32.1)
5600 West / 13400 South	Signal	(4)		×	50.8	D	-
5000 West / 13400 South	Signal	<i>3</i>		ě	30.4	С	5
8000 West / Herriman Boulevard	NB/SB Stop	SB	10.4	В	3 €		ŧ
7300 West / Herriman Boulevard	Signal			*	>120.0	F	C (31.6)
6800 West / Herriman Boulevard	Signal	- 38		•	36.2	Đ	
Silver Sky Drive / 6400 West	EB/WB Stop	EB	24.4	С	<u>j.</u>	1.81	

1. This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.



D. Queuing Analysis

Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - Northbound Approach 410 feet (a.m. peak), 525 feet (p.m. peak)
- 7300 West / 11800 South
 - Northbound Approach 420 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach 475 feet (p.m. peak)
- 6400 West / 11800 South
 - Northbound Approach 485 feet (p.m. peak)
 - Southbound Approach 780 feet (p.m. peak)
 - Eastbound Approach 610 feet (p.m. peak)
 - Westbound Approach 410 feet (p.m. peak)
- 6400 West / Herriman Boulevard
 - Northbound Approach >1,000 feet (p.m. peak)
 - Eastbound Approach 395 feet (p.m. peak)
 - Westbound Approach 355 feet (p.m. peak)
- 6000 West / Herriman Boulevard
 - Westbound Approach 370 feet (a.m. peak)
- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach 735 feet (a.m. peak)
 - Southbound Approach 700 feet (p.m. peak)
 - Eastbound Approach 365 feet (a.m. peak), 460 feet (p.m. peak)
 - Westbound Approach 635 feet (a.m. peak), 400 feet (p.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Eastbound Approach >1,000 feet (a.m. peak)
 - Westbound Approach 545 feet (p.m. peak)
- 6800 West / Herriman Highway
 - Eastbound Approach 610 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Westbound Approach 515 feet (p.m. peak)
- 7300 West / Herriman Highway
 - Southbound Approach >1,000 feet (a.m. and p.m. peak)
 - Eastbound Approach 665 feet (p.m. peak)
 - Westbound Approach 415 feet (a.m. peak), 865 feet (p.m. peak)
- 6400 West / Main Street
 - Northbound Approach 430 feet (a.m. peak)
 - Southbound Approach 670 feet (a.m. peak), 675 feet (p.m. peak)



- Eastbound Approach >1,000 feet (p.m. peak)
- Westbound Approach 395 feet (p.m. peak)
- 5600 West / Main Street
 - Northbound Approach 590 feet (a.m. peak), 725 feet (p.m. peak)
 - Southbound Approach 540 feet (p.m. peak)
- 6400 West / 13400 South
 - Northbound Approach 545 feet (a.m. peak)
 - Southbound Approach 375 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 5600 West / 13400 South
 - Eastbound Approach 455 feet (a.m. peak), 530 feet (p.m. peak)
 - Westbound Approach 880 feet (p.m. peak)
- 5000 West / 13400 South
 - Southbound Approach >1,000 feet (a.m. peak)
 - Westbound Approach 530 feet (p.m. peak)
- 7300 West / Herriman Boulevard
 - Southbound Approach 475 feet (a.m. peak), 905 feet (p.m. peak)
 - Eastbound Approach 390 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Westbound Approach 415 feet (a.m. peak), >1,000 feet (p.m. peak)
- 6800 West / Herriman Boulevard
 - Eastbound Approach 510 feet (a.m. peak), 650 feet (p.m. peak)
 - Westbound Approach 620 feet (a.m. peak), >1,000 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.

E. Mitigation Measures

It is recommended that permissive/protected left-turn phasing be installed on all approaches to the 7300 West / 11800 South, 6400 West / 11800 South, 7300 West / Herriman Boulevard, 6400 West / Herriman Boulevard, and 6400 West / Main Street intersections.

It is also recommended that dual left-turn lanes be constructed on the northbound approach to the 6400 West / 11800 South intersection and on the westbound approach to the 6400 West / 13400 South intersection.

It is also anticipated that 7300 West will need to be expanded to a five-lane cross section north of Herriman Boulevard to accommodate the projected traffic volumes.



Hales Engineering analyzed a mitigated scenario which assumed that these recommended mitigation measures had been implemented. Based on this analysis the recommended mitigation measures are anticipated to result in acceptable levels of service throughout the study area.

No additional mitigation measures are recommended.



X. FUTURE (2042) BACKGROUND CONDITIONS

A. Purpose

The purpose of the future (2042) background analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions. Through this analysis, future background traffic operational deficiencies can be identified, and potential mitigation measures recommended.

B. Roadway Network

According to the WFRC Regional Transportation Plan, 7300 West is planned to be expanded to a five-lane cross section south of Herriman Highway as Phase 2 (2031-2040) project. It was assumed that this improvement, as well as all previously recommended background mitigation measures, had been completed prior to 2042.

C. Traffic Volumes

Hales Engineering obtained future (2042) forecasted volumes from a modified version of the WFRC / MAG TDM. This version of the WFRC/MAG TDM was tailored specifically for this project by Horrocks Engineers (and reviewed by Salt Lake County) to forecast future average weekday daily traffic (AWDT) volumes within the study area. Peak period turning movement counts were estimated using National Cooperative Highway Research Program (NCHRP) 255 methodologies which utilize existing peak period turn volumes and future AWDT volumes to project the future turn volumes at the major intersections. Future (2042) morning and evening peak hour turning movement volumes are shown in Figure 28 and Figure 29.

D. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at LOS E or LOS F in future (2042) background conditions as shown in Table 19 and Table 20:

- 6400 West / 11800 South (Evening Peak)
- 7300 West / Herriman Boulevard (Evening Peak)
- Anthem Park Boulevard / Herriman Boulevard (Morning Peak)
- Main Street / Herriman Boulevard / 12600 South (Morning and Evening Peak)
- 6400 West / Main Street (Evening Peak)
- 6400 West / 13400 South (Evening Peak)



These results serve as a baseline condition for the impact analysis of the proposed development for future (2042) conditions.

E. Queuing Analysis

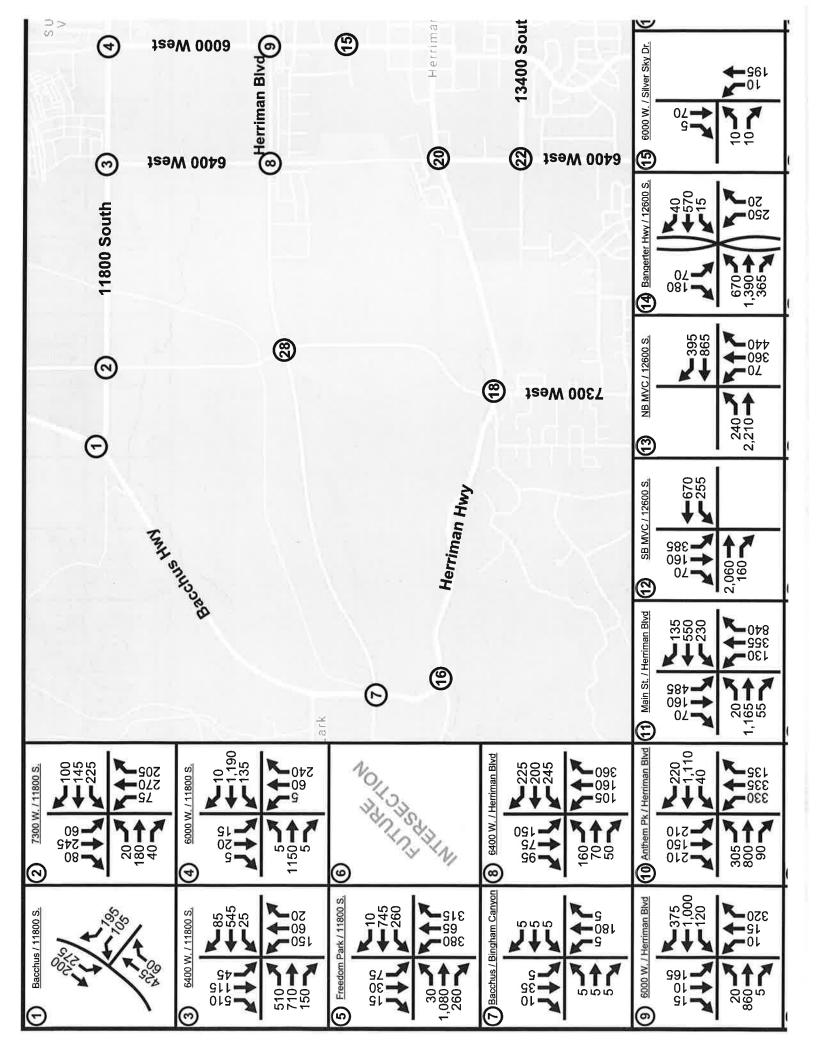
Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - Northbound Approach 720 feet (a.m. peak), 495 feet (p.m. peak)
- 7300 West / 11800 South
 - o Northbound Approach 360 feet (a.m. peak), 655 feet (p.m. peak)
 - Southbound Approach 855 feet (p.m. peak)
 - Westbound Approach 825 feet (p.m. peak)
- 6400 West / 11800 South
 - Southbound Approach 840 feet (p.m. peak)
 - Eastbound Approach 415 feet (p.m. peak)
 - Westbound Approach 460 feet (p.m. peak)
- Freedom Park Drive / 11800 South
 - Northbound Approach 410 feet (p.m. peak)
 - Westbound Approach 445 feet (p.m. peak)
- 7300 West / Herriman Boulevard
 - Northbound Approach 425 feet (p.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
 - Eastbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach 565 feet (p.m. peak)
- 6400 West / Herriman Boulevard
 - Northbound Approach 375 feet (p.m. peak)
- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach >1,000 feet (a.m. peak)
 - Southbound Approach 460 feet (a.m. peak), 635 feet (p.m. peak)
 - Westbound Approach 925 feet (a.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Northbound Approach 990 feet (a.m. peak), 385 feet (p.m. peak)
 - Southbound Approach >1,000 feet (a.m. peak), 780 feet (p.m. peak)
 - Eastbound Approach 355 feet (a.m. peak), 605 feet (p.m. peak)
 - Westbound Approach 765 feet (p.m. peak)
- 7300 West / Herriman Highway
 - Southbound Approach 360 feet (a.m. peak)
 - Westbound Approach 390 feet (p.m. peak)



- 6400 West / Main Street
 - Northbound Approach 505 feet (p.m. peak)
 - Southbound Approach 905 feet (p.m. peak)
 - Eastbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach 750 feet (p.m. peak)
- 5600 West / Main Street
 - Northbound Approach 610 feet (a.m. peak), 445 feet (p.m. peak)
 - Southbound Approach 460 feet (p.m. peak)
- 6400 West / 13400 South
 - Northbound Approach 765 feet (a.m. peak)
 - Southbound Approach ->1,000 feet (p.m. peak)
 - Westbound Approach 565 feet (p.m. peak)
- 5600 West / 13400 South
 - Westbound Approach 815 feet (p.m. peak)
- 5000 West / 13400 South
 - Southbound Approach >1,000 feet (a.m. and p.m. peak)
 - Eastbound Approach 395 feet (a.m. peak)
 - Westbound Approach 720 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.



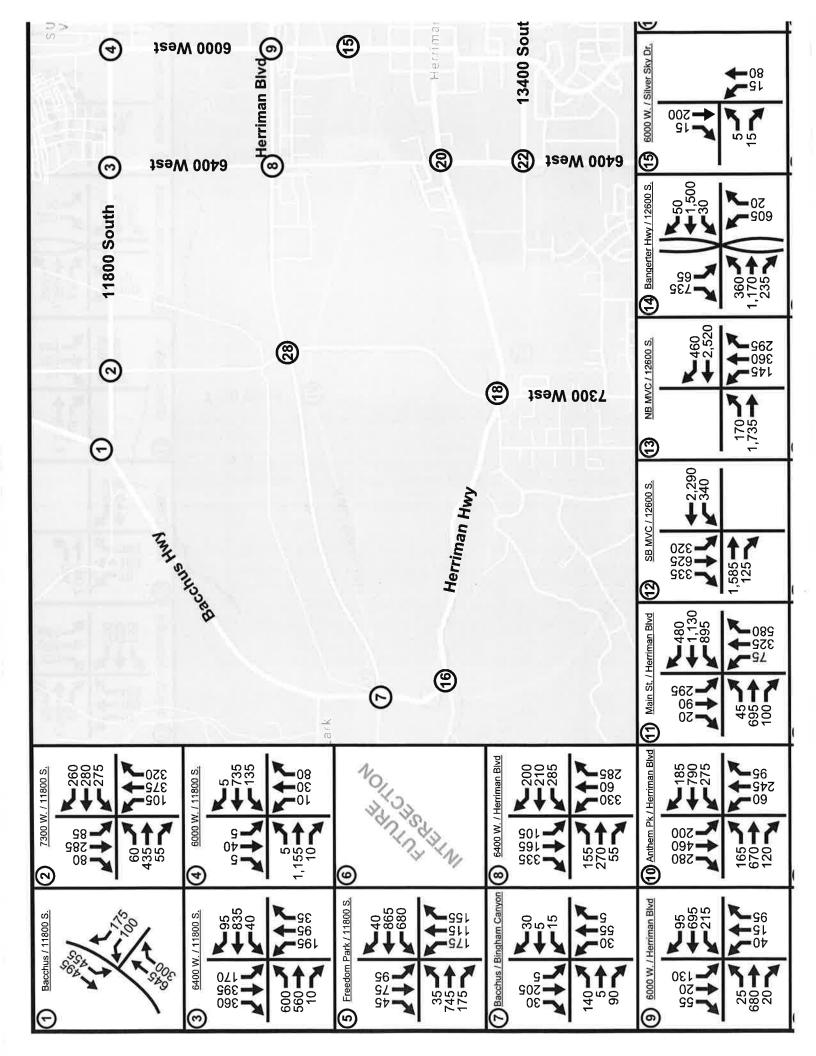




Table 19: Future (2042) Background Morning Peak Hour Level of Service

Intersection		Worst Approach Overall Intersection					Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay	LOS ¹	Aver. Delay	LOS ²	LOS (Delay)
Description	Control	Арргоасп	(Sec/Veh)	LU3	(Sec/Veh) ²	LUS	LOS (DEIBY)
Bacchus Highway / 11800 South	Signal				42.2	D	
7300 West / 11800 South	Signal	=	(C	-	19.8	В	ä
6400 West / 11800 South	Signal	•		(#E	22.2	С	
6000 West / 11800 South	Signal	≅	196	*	15.1	В	
Freedom Park Drive / 11800 South	Signal	•		550	28.5	С	-
Bingham Canyon Mine /	EB/WB	ЕВ	4.4	٨			
Bacchus Highway	Stop	EB	4.4	Α		•	
7300 West / Herriman Boulevard	Signal	16	-	35 Yi	20.9	С	•
6400 West / Herriman Boulevard	Signal	*		528	16.8	В	
6000 West /	Signal				14.0	n	_
Herriman Boulevard	Signal			(#)	14.9	В	
Anthem Park Boulevard /	Cignal				61.5	Е	D (45.6)
Herriman Boulevard	Signal				01.5	E	D (45.6)
Main Street / Herriman Boulevard /	Cianal				75.0		C (24.7)
12600 South	Signal			:=::	75.0	E	C (34.7)
SB MVC / 12600 South	Signal	2 🚾		· **	=	3	=
NB MVC / 12600 South	Signal		2		-		
Bangerter Highway / 12600 South	Signal			14 9		-	-
Silver Sky Drive / 6000 West	EB Stop	EB	4.1	Α	-	-	
Butterfield Canyon Road /							
Herriman Highway /	EB Stop	EB	7.2	A		•	
Bacchus Highway							
7300 West / Herriman Highway	Signal	(E			21.0	С	
6400 West / Main Street	Signal	Q#:	2€2	~ 5	27.0	С	
5600 West / Main Street	Signal		<u></u>	÷.v	22.6	С	
6400 West / 13400 South	Signal	>>	: # E	20	29.9	С	
5600 West / 13400 South	Signal	(e)		÷.	26.7	С	8
5000 West / 13400 South	Signal	0.00	5 ₩ 3	* :	46.6	D	
SB MVC / 13400 South	Signal	18	*		7	5	
NB MVC / 13400 South	Signal	::e:		541	*	÷	

^{1.} This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.



Table 20: Future (2042) Background Evening Peak Hour Level of Service

Intersection		Wor	st Approach	- i	Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh)	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal				26.4	С	.*
7300 West / 11800 South	Signal				45.2	D	1,50
6400 West / 11800 South	Signal	(#)	-		56.4	E	D (54.7)
6000 West / 11800 South	Signal			*	12.9	В). *
Freedom Park Drive / 11800 South	Signal				30.0	С	3€3
Bingham Canyon Mine /	EB/WB	EB	6.3	А		-	045
Bacchus Highway	Stop	ED	0.5	Α	12		
7300 West / Herriman Boulevard	Signal			•	>120.0	F	C (31.3)
6400 West / Herriman Boulevard	Signal	i=:	8		18.8	В	K=:
6000 West /	Signal	221	12	2	13.8	В	
Herriman Boulevard	Signal				15.6		
Anthem Park Boulevard /	Signal	Sec. 1	12		32.4	С	
Herriman Boulevard	Signal				32.4		
Main Street / Herriman Boulevard /	Signal	(42)	2		66.8	E	C (30.7)
12600 South	Signal				00.8		C (30.7)
SB MVC / 12600 South	Signal	•	ê	-	ě	٠	-
NB MVC / 12600 South	Signal		-			565	
Bangerter Highway / 12600 South	Signal	96	Œ.	3	Ĕ	-	2
Silver Sky Drive / 6000 West	EB Stop	EB	3.5	Α	-	1591	
Butterfield Canyon Road /							
Herriman Highway /	EB Stop	EB	4.3	A	•	(*	-
Bacchus Highway							
7300 West / Herriman Highway	Signal	<u>9€</u> :		*	22.8	С	
6400 West / Main Street	Signal	Y SET	ia i	•	>120.0	F	C (29.3)
5600 West / Main Street	Signal	196	· ·		21.6	С	=
6400 West / 13400 South	Signal	Ē	1911		93.1	F	C (34.0)
5600 West / 13400 South	Signal		(#Z)	357	45.9	D	
5000 West / 13400 South	Signal	i i	121	141	43.7	D	
SB MVC / 13400 South	Signal	Ŀ		27.1			
NB MVC / 13400 South	Signal		127	*	2	ě	- 4

This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.
 This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.
 SB = Southbound approach, etc.



F. Mitigation Measures

It is anticipated that additional capacity will be needed at the following intersections to accommodate the projected 2042 traffic conditions:

- 6400 West / 11800 South
- 7300 West / Herriman Boulevard
- Anthem Park Boulevard / Herriman Boulevard
- Main Street / Herriman Boulevard / 12600 South
- 6400 West / Main Street
- 6400 West / 13400 South

The following mitigation measures are recommended:

- 6400 West / 11800 South
 - Add right-turn lanes to the east- and westbound approaches
- 7300 West / Herriman Boulevard
 - Add right-turn lanes to all approaches
 - o Install permissive/protected left-turn phasing on all approaches
- Anthem Park Boulevard / Herriman Boulevard
 - Add right-turn lanes to the east- and westbound approaches
- Main Street / Herriman Boulevard / 12600 South
 - Add second through lane to the northbound approach
 - o Increase left-turn storage length on the southbound approach
 - Construct dual left-turn lanes on the westbound approach
- 6400 West / Main Street
 - Add right-turn lanes to the east- and westbound approaches
- 6400 West / 13400 South
 - o Increase left-turn storage length on the southbound approach
 - Construct dual left-turn lanes on the westbound approach

Hales Engineering analyzed a mitigated scenario which assumed that these recommended mitigation measures had been implemented. Based on this analysis the recommended mitigation measures are anticipated to result in acceptable levels of service throughout the study area.

No additional mitigation measures are recommended.



XI. FUTURE (2042) PLUS PROJECT CONDITIONS

A. Purpose

The purpose of the future (2042) plus project analysis is to study the intersections and roadways during the peak travel periods of the day for future background traffic and geometric conditions plus the net trips generated by the proposed development. This scenario provides valuable insight into the potential impacts of the proposed project on future background traffic conditions.

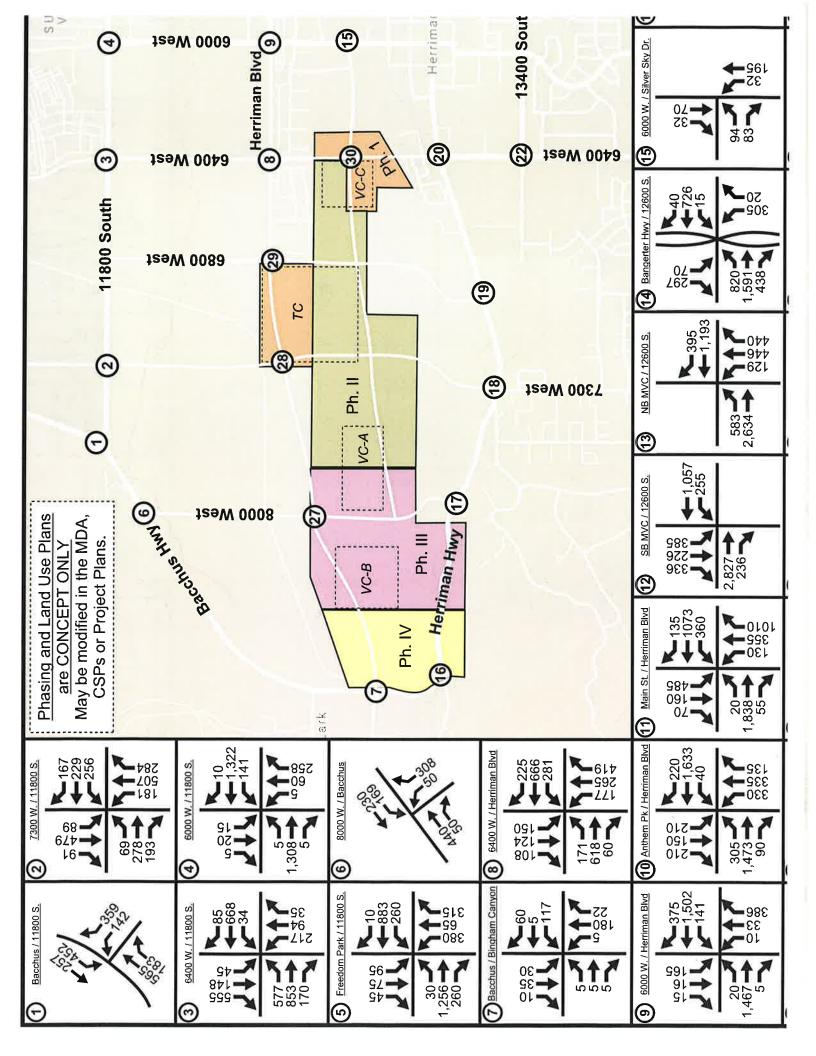
B. Traffic Volumes

Hales Engineering added the Phase IV project trips discussed in Chapter V to the future (2042) background traffic volumes to predict turning movement volumes for future (2042) plus project conditions. Additional turning movement volumes were added manually to new project roadways as well to match better with the volumes provided by Horrocks in the build travel demand models. Future (2042) plus project evening peak hour turning movement volumes are shown in Figure 30 and Figure 31.

C. Level of Service Analysis

Hales Engineering determined that the following intersections are anticipated to operate at levels of service E or F in future (2042) plus project conditions as shown in Table 21 and Table 22:

- 7300 West / 11800 South (Evening Peak)
- 6400 West / 11800 South (Evening Peak)
- 8000 West / Bacchus Highway (Evening Peak)
- Anthem Park Boulevard / Herriman Boulevard (Morning and Evening Peak)
- Main Street / Herriman Boulevard (Morning Peak)
- 8000 West / Herriman Highway (Evening Peak)
- 7300 West / Herriman Highway (Morning and Evening Peak)
- 6400 West / Main Street (Evening Peak)
- 5600 West / 13400 South (Evening Peak)
- 8000 West / Herriman Boulevard (Evening Peak)
- 7300 West / Herriman Boulevard (Evening Peak)



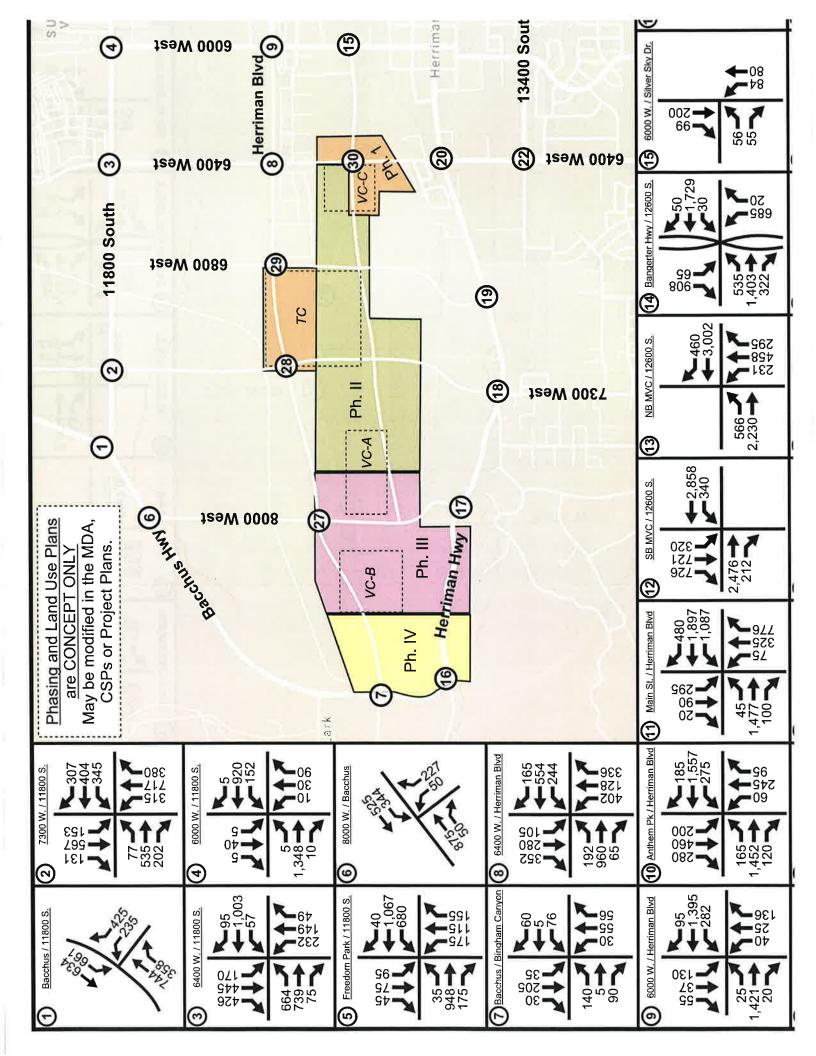




Table 21: Future (2042) Plus Project Morning Peak Hour Level of Service

Intersection		Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal	-	- 4		30.2	С	ě
7300 West / 11800 South	Signal	=	195	#	27.3	С	•
6400 West / 11800 South	Signal	<u> </u>	72	E	26.0	С	4
6000 West / 11800 South	Signal		(#)		16.3	В	
Freedom Park Drive / 11800 South	Signal	9	-	ě	29.7	С	
8000 West / Bacchus Highway	NB Stop	NB	15.5	С	oje:	3 + 3	18
Bingham Canyon Mine /	EB/WB	WB	4.9	Α	949	: <u>`</u>	i i
Bacchus Highway	Stop						-
6400 West / Herriman Boulevard	Signal	9	<u> </u>	15	31.7	С	
6000 West / Herriman Boulevard	Signal	5 .		×	42.8	D	X 5 5
Anthem Park Boulevard / Herriman Boulevard	Signal	4 ,	255	S-#	102.7	F	D (39.3)
Main Street / Herriman Boulevard / 12600 South	Signal	50	3.5) (=)	75.9	E	D (39.2)
Silver Sky Drive / 6000 West	EB Stop	EB	5.8	Α	· · · · · · · · · · · · · · · · · · ·	9±7	74
Butterfield Canyon Road /							
Herriman Highway /	EB Stop	EB	3.8	Α	-	-	-
Bacchus Highway							
8000 West / Herriman Highway	SB Stop	SB	21.5	С	•	•	<u>(#)</u>
7300 West / Herriman Highway	Signal	₩	(*)	ne:	82.3	F	C (33.5)
6800 West / Herriman Highway	Signal	=	8	2	21.5	С	
6400 West / Main Street	Signal	=	3#0		41.9	D	
5600 West / Main Street	Signal			(22.2	С	
6400 West / 13400 South	Signal				32.0	С	:: * :
5600 West / 13400 South	Signal	<u> </u>	34		33.0	С	<u></u>
5000 West / 13400 South	Signal	- 5	Œ	()⊕(29.0	С	3#)
8000 West / Herriman Boulevard	NB/SB	SB	16.0	С	=	121	
	Stop						
7300 West / Herriman Boulevard	Signal			-	33.2	С	
6800 West / Herriman Boulevard	Signal	*		55 4 5	37.6	D	
Silver Sky Drive / 6400 West	EB/WB Stop	EB	29.1	D	×	-	3

¹ This represents the worst approach LOS and delay (seconds i vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds i vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.

Source: Hales Engineering, November 2019



Table 22: Future (2042) Plus Project Evening Peak Hour Level of Service

Intersection		Wor	st Approach		Overall Inters	ection	Mitigated
Description	Control	Approach ^{1,3}	Aver. Delay (Sec/Veh) ¹	LOS ¹	Aver. Delay (Sec/Veh) ²	LOS ²	LOS (Delay)
Bacchus Highway / 11800 South	Signal		35	<u>, e</u>	32.2	С	
7300 West / 11800 South	Signal	¥	-	74	63.4	E	D (41.2)
6400 West / 11800 South	Signal	×	Se		103.5	F	D (42.1)
6000 West / 11800 South	Signal	9	<u> </u>	74	13.2	В	
Freedom Park Drive / 11800 South	Signal	- 4	3(€3	<u>:</u>	26.3	С	0°E
8000 West / Bacchus Highway	NB Stop	NB	>75	F		(4)	C (31.3)
Bingham Canyon Mine / Bacchus Highway	EB/WB Stop	ЕВ	6.0	Α	(2)	S#3:	:#
6400 West / Herriman Boulevard	Signal		1981	14	37.5	D	
6000 West / Herriman Boulevard	Signal	ü	121	(E)	21.2	С	14
Anthem Park Boulevard / Herriman Boulevard	Signal	ä	14	140	65.1	E	D (38.9)
Main Street / Herriman Boulevard / 12600 South	Signal	<u>u</u>	· eg	Щ	53.4	D	¥
Silver Sky Drive / 6000 West	EB Stop	EB	6.1	Α	% = :	899	
Butterfield Canyon Road / Herriman Highway / Bacchus Highway	EB Stop	ЕВ	3.9	Α	ASS.	6 5 1	__
8000 West / Herriman Highway	SB Stop	SB	>75	F	K#		C (21.4)
7300 West / Herriman Highway	Signal	-	#	-	93.8	F	C (30.5)
6800 West / Herriman Highway	Signal	⊕ 1	*		26.7	D	. *
6400 West / Main Street	Signal	3	Ë	10	61.3	Е	D (41.1)
5600 West / Main Street	Signal	(→);	-	*	34.1	С	
6400 West / 13400 South	Signal	3	ê	590	51.9	D	
5600 West / 13400 South	Signal	(= ())	*	*	73.1	Е	D (47.2)
5000 West / 13400 South	Signal	3 9)		8	40.6	D	ě
8000 West / Herriman Boulevard	NB/SB Stop	NB	>75	F		T:	C (21.4)
7300 West / Herriman Boulevard	Signal	(* ()	¥		107.9	F	C (34.4)
6800 West / Herriman Boulevard	Signal	<u>:</u>	÷.	5	41.5	D	9
Silver Sky Drive / 6400 West	EB/WB Stop	EB	25.7	С	÷	=	ភ

1. This represents the worst approach LOS and delay (seconds / vehicle) and is only reported for non-all-way stop unsignalized intersections.

2. This represents the overall intersection LOS and delay (seconds / vehicle) and is reported for all-way stop and signal-controlled intersections.

3. SB = Southbound approach, etc.



D. Queuing Analysis

Hales Engineering calculated the 95th percentile queue lengths for each of the study intersections. Notable 95th percentile queues are listed below:

- Bacchus Highway / 11800 South
 - o Northbound Approach 445 feet (a.m. peak), 625 feet (p.m. peak)
 - Southbound Approach 400 feet (p.m. peak), 385 feet (p.m. peak)
- 7300 West / 11800 South
 - Northbound Approach 370 feet (p.m. peak)
 - Southbound Approach 390 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Eastbound Approach 570 feet (p.m. peak)
 - Westbound Approach 500 feet (p.m. peak)
- 6400 West / 11800 South
 - Northbound Approach 385 feet (p.m. peak)
 - Southbound Approach 815 feet (p.m. peak
 - Eastbound Approach 535 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 6400 West / Herriman Boulevard
 - Northbound Approach 490 feet (p.m. peak)
 - Southbound Approach 485 feet (p.m. peak)
 - Eastbound Approach 485 feet (p.m. peak)
- 6000 West / Herriman Boulevard
 - Northbound Approach 575 feet (a.m. peak)
 - Eastbound Approach >1,000 feet (a.m. peak)
 - Westbound Approach 455 feet (a.m. peak)
- Freedom Park Drive / 11800 South
 - Northbound Approach 360 feet (a.m. peak)
 - Westbound Approach 390 feet (p.m. peak)
- 8000 West / Bacchus Highway
 - Northbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 6400 West / Herriman Boulevard
 - Northbound Approach 490 feet (p.m. peak)
 - Southbound Approach 485 feet (p.m. peak)
 - Eastbound Approach 485 feet (p.m. peak)
- 6000 West / Herriman Boulevard
 - Northbound Approach 575 feet (a.m. peak)
 - Eastbound Approach >1,000 feet (a.m. peak)
 - Westbound Approach 455 feet (a.m. peak)



- Anthem Park Boulevard / Herriman Boulevard
 - Northbound Approach >1,000 feet (a.m. peak)
 - o Southbound Approach 730 feet (a.m. peak), 680 feet (p.m. peak)
 - Eastbound Approach >1,000 feet (a.m. peak), 660 feet (p.m. peak)
 - Westbound Approach >1,000 feet (a.m. and p.m. peak)
- Main Street / Herriman Boulevard / 12600 South
 - Eastbound Approach >1,000 feet (a.m. and p.m. peak)
 - Westbound Approach 365 feet (a.m. peak), 755 feet (p.m. peak)
- 8000 West / Herriman Highway
 - Southbound Approach 735 feet (p.m. peak)
 - Eastbound Approach 540 feet (p.m. peak)
- 7300 West / Herriman Highway
 - Southbound Approach >1,000 feet (a.m. and p.m. peak)
 - o Eastbound Approach 450 feet (a.m. peak), 925 feet (p.m. peak)
 - Westbound Approach 370 feet (a.m. peak), 525 feet (p.m. peak)
- 6800 West / Herriman Highway
 - Eastbound Approach 605 feet (a.m. peak), 535 feet (p.m. peak)
 - Westbound Approach 445 feet (a.m. peak), 600 feet (p.m. peak)
- 6400 West / Main Street
 - Northbound Approach >1,000 feet (a.m. peak), 905 feet (p.m. peak)
 - Southbound Approach 605 feet (a.m. peak), 960 feet (p.m. peak)
 - Eastbound Approach >1,000 feet (p.m. peak)
 - Westbound Approach 695 feet (p.m. peak)
- 5600 West / Main Street
 - Northbound Approach 540 feet (a.m. peak), 810 feet (p.m. peak)
 - Southbound Approach 610 feet (p.m. peak)
 - Westbound Approach 370 feet (p.m. peak)
- 6400 West / 13400 South
 - Northbound Approach 820 feet (a.m. peak)
 - Southbound Approach >1,000 feet (p.m. peak)
- 5600 West / 13400 South
 - Eastbound Approach 570 feet (a.m. peak), 620 feet (p.m. peak)
 - Westbound Approach >1,000 feet (p.m. peak)
- 5000 West / 13400 South
 - Southbound Approach 970 feet (a.m. peak), >1,000 feet (p.m. peak)
 - Westbound Approach 410 feet (p.m. peak)
- 7300 West / Herriman Boulevard
 - Northbound Approach 375 feet (p.m. peak)
 - Southbound Approach 480 feet (a.m. peak), >1,000 feet (p.m. peak)



- Eastbound Approach 390 feet (a.m. peak), >1,000 feet (p.m. peak)
- Westbound Approach 385 feet (a.m. peak), >1,000 feet (p.m. peak)
- 6800 West / Herriman Highway
 - Eastbound Approach 605 feet (a.m. peak), 535 feet (p.m. peak)
 - Westbound Approach 445 feet (a.m. peak), 600 feet (p.m. peak)

Detailed queueing reports are included in Appendix E.

E. Mitigation Measures

At the 7300 West / 11800 South intersection, it is recommended that dual left-turn lanes be installed on the north- and westbound approaches when warranted.

At the 6400 West / 11800 South intersection, it is recommended that a southbound right-turn overlap phase be used, that a southbound through lane be added, and that the northbound right-turn lane be converted into a shared through-right.

It is recommended that 6400 West be widened to a five-lane cross-section between 11800 South and Herriman Boulevard to provide needed capacity on the roadway and nearby intersections.

At the 8000 West / Bacchus Highway intersection, it is anticipated that the volumes will warrant a traffic signal with future (2042) plus project conditions. It is recommended that a signal be installed when warranted with turn pockets.

At the Anthem Park Boulevard / Herriman Boulevard intersection, it is recommended that dual left-turn lanes be installed on the north- and westbound approaches, that the northbound left-turn storage be extended, and that the northbound and southbound right-turn lanes be converted into shared through-right lanes.

At the Main Street / Herriman Boulevard intersection, it is recommended that the eastbound left-turn signal phase be changed to the lagging phase behind the westbound through phase. It is also recommended that a separate eastbound right-turn lane be added and that a westbound through lane be added.

It is recommended that Herriman Boulevard between Main Street and 6000 West be widened to a seven-lane cross-section to accommodate the high traffic volumes.

At the 8000 West / Herriman Highway intersection, it is anticipated that the volumes will warrant a traffic signal with future (2042) plus project conditions. It is recommended that a signal be installed when warranted with turn pockets.



At the 7300 West / Herriman Highway intersection, it is recommended that a southbound right-turn lane be added, that permissive-protected phasing be implemented on all approaches, and that dual left-turns be installed on the southbound approach.

At the 6400 West / Main Street intersection, it is recommended that dual left-turns be added on the northbound approach and that the eastbound right-turn be channelized.

At the 5600 West / 13400 South intersection, it is recommended that dual left-turns be added on the eastbound approach, that the eastbound and westbound through phases be assigned as lagging phases, and that the eastbound right-turn lane be converted into a shared through-right.

At the 8000 West / Herriman Boulevard intersection, it is anticipated that the volumes will warrant a traffic signal with future (2042) plus project conditions. It is recommended that a signal be installed when warranted with turn pockets.

At the 7300 West / Herriman Boulevard intersection, it is recommended that dual left-turns be added to the south-, east-, and westbound approaches, that the westbound right-turn be channelized, that a northbound and southbound lane be added, and that the eastbound right-turn lane be converted into a shared through-right lane.

It is recommended that Herriman Boulevard be widened to a five-lane cross-section between 7300 West and 6800 West to accommodate the high traffic volumes.

Hales Engineering completed a mitigated scenario with the proposed improvements. It is anticipated that all study intersections will operate at acceptable levels of service with the proposed improvements.



APPENDIX A

Turning Movement Counts



APPENDIX B

Project Phasing Plan



APPENDIX C

Trip Generation



APPENDIX D

LOS Reports



APPENDIX E

95th Percentile Queue Length Reports



APPENDIX F

Recommended Improvements



Page 1 of 4

MEMORANDUM

Date:

December 12, 2019

To:

Salt Lake County

From:

Hales Engineering

Subject:

Salt Lake County - Olympia Hills TIS Addendum

UT19-1472

This memorandum discusses the trip generation for the proposed Olympia Hills development in Salt Lake County, Utah. This memorandum serves as an addendum to the traffic impact study (TIS) that was completed in December 2019.

Background

Since the TIS has been completed, additional details regarding land uses have been determined for the project. It was determined that the project will include more single-family housing than originally proposed and some senior housing. The TIS assumed that all multi-family would be low-rise housing (1 to 2 stories). However, with additional details provided, the multi-family housing was broken up into low-rise (1 to 2 stories) and mid-rise (3+ stories) as each generates different trip numbers according to the Institute of Transportation Engineering (ITE). A comparison of the land uses in the TIS with the refined land uses are shown in Table 1. As identified, the total number of dwelling units and the total square footage of office and retail was kept the same.

Table 1: Land Use Comparison

17 - 50	Land Use	Original TIS	Refined Land Uses	Δ
	Single-family	950 DU	1,480 DU	+ 530 DU
	Multi-family (Low-Rise)	5,380 DU	862 DU	- 4,518 DU
	Multi-family (Mid-Rise)	-	3,269 DU	+ 3,269 DU
Residential	Senior Housing – Detached	_	425 DU	+ 425 DU
	Senior Housing - Attached	-	294 DU	+ 294 DU
	TOTAL	6,330 DU	6,330 DU	-
	Office	1,394,000 sf	1,394,000 sf	-
	Retail	381,000 sf	381,000 sf	-



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Trip Generation

Trip generation for the development was calculated using trip generation rates published in the ITE *Trip Generation (10th Edition, 2017)*. Detailed trip generation sheets for both the original TIS and the refined land uses are provided in Appendix A and Appendix B, respectively. Hales Engineering recalculated the internal capture rates for the Town Center and Village Centers based on the refined trip generation as well. Those sheets are also found in Appendix B.

The trip generation of the original TIS compared with the refined trip generation is shown in Table 2. As identified, the refined land uses have a lower daily trip generation than the uses in the original TIS; however, the peak hour trip generation is slightly higher with the refined land uses. Although the refined peak hour trips are a little higher when compared to the original TIS, it is not anticipated that the additional trips will impact the results and recommendations of the TIS.

Trip Generation Original TIS Refined LU Δ Weekday Daily 76,182 68,640 -7,542Morning Peak Hour 4,472 4,535 63 **Evening Peak Hour** 6,009 234 5,775

Table 2: Trip Generation Comparison

Conclusions

The key findings are as follows:

- The Olympia Hills land uses were refined to a more realistic scenario for the project. More single-family homes were included in the refined land uses as well as some senior housing. The type of multi-family dwelling units was also refined.
- It is anticipated that the refined land uses will generate approximately 7,542 less daily trips, 63 additional morning peak hour trips, and 234 additional evening peak hour trips.
 - Although the refined peak hour trips are a little higher when compared to the original TIS, it is not anticipated that the additional trips will impact the results and recommendations of the TIS.

APPENDIX A

TIS Trip Generation

					ke County Generation			IS						
Weekd	ay Dai	ly	# of	Unit	Тпр	%	%	Tríps	Trips	Internal	Transit	Net Trips	Net Trips	Total Daily
Phase	Area	Land Use	Units	Туре	Generation		Exiting	Entering	Exiting	Capture ⁷	Reduction*	Entering	Exiting	Trips
1 & 2	TC TC	Multifamily Housing (Low-Rise) (220) Single-Family Detached Housing (210)	795 119	Dwelling Units Dwelling Units	5,970 1,222	50% 50%	50% 50%	2,985 611	2,985 611	0% 0%	2.5%	2,910 596	2,910 596	5,820 1,192
1 & 2	TC	General Office Building (710)	1272	1,000 Sq. Ft, GFA	12,506	50%	50%	6,253	6,253	0%	2.5%	6,097	6,097	12,194
1 & 2	TC	Shopping Center (820)	258.8	1,000 Sq. Ft. GLA	9,770	50%	50%	4,885	4,885	0%	2.5%	4,763	4,763	9,526
1 & 2	VC-C	Multifamily Housing (Low-Rise) (220)	498	Dwelling Units	3,726	50%	50%	1,863	1,863	0%	2,5%	1,816	1,816	3,632
1 & 2	VC-Ç	Single-Family Detached Housing (210)	78	Dwelling Units	828	50%	50%	414	414	0%	2.5%	404	404	808
1 & 2	VC-C	General Office Building (710)	31.9	1,000 Sq. Ft. GFA	352	50%	50%	176	176	0%	2.5%	172	172	344
1 & 2	VC-C	Shopping Center (820)	36.3	1,000 Sq. Ft, GLA	1,372	50%	50%	686	686	0%	2,5%	669	669	1,338
1	Olher	Multifamily Housing (Low-Rise) (220)	573	Dwelling Units	4,292	50%	50%	2,146	2,146	0%	2.5%	2,092	2,092	4,184
1	Other	Single-Family Detached Housing (210)	119	Dwelling Units	1,222	50%	50%	611	611	0%	2.5%	596	596	1,192
2 & 3	VC-A	Multifamily Housing (Low-Rise) (220)	570	Dwelling Units	4,270	50%	50%	2,135	2,135	0%	2,5%	2,082	2,082	4,164
2 & 3	VC-A	Single-Family Detached Housing (210)	60	Dwelling Units	650	50%	50%	325	325	0%	2.5%	317	317	634
2 & 3 2 & 3	VC-A VC-A	General Office Building (710)	90.1 45.4	1,000 Sq. Ft. GFA	960 1,714	50% 50%	50% 50%	480 857	480 857	0% 0%	2,5% 2.5%	468 836	468 836	936 1,672
2 0.3	Olher	Shopping Center (820) Multifamily Housing (Low-Rise) (220)	486	1,000 Sq. Ft. GLA Dwelling Units	3,634	50%	50%	1,817	1,817	0%	2.5%	1,772	1,772	3,544
2	Other	Single-Family Detached Housing (210)	369	Dwelling Units	3,458	50%	50%	1,729	1,729	0%	2,5%	1,686	1,686	3,372
3	VC-B	Multifamily Housing (Low-Rise) (220)	900	Dwelling Units	6,764	50%	50%	3,382	3,382	0%	2,5%	3,297	3,297	6,594
3	VC-B	Single-Family Delached Housing (210)	72	Dwelling Units	770	50%	50%	385	385	0%	2,5%	375	375	750
3	VC-B	Shopping Center (820)	40.5	1,000 Sq. Ft, GLA	1,530	50%	50%	765	765	0%	2,5%	746	746	1,492
3	Other	Multifarnily Housing (Low-Rise) (220)	449	Dwelling Units	3,354	50%	50%	1,677	1,677	0%	2.5%	1,635	1,635	3,270
3	Other	Single-Family Detached Housing (210)	43	Dwelling Units	480	50%	50%	240	240	0%	2.5%	234	234	468
4	Other	Multifamlly Housing (Low-Rise) (220)	1109	Dwelling Units	8,344	50%	50%	4,172	4,172	0%	2.5%	4,068	4,068	8,136
4	Other	Single-Family Detached Housing (210)	90	Dwelling Units	944	50%	50%	472	472	0%	2.5%	460	460	920
		Project Total Daily Trips		10000	78,132			39,066	39,066			38,091	38,091	76,182
Mornin			# of	Unit	Trip			Trips	Pripa	Internal	(ranu)	Net Trips	Net Trips	Total a.m.
Phase	Area	Land Use	Unifs	Type	Generation		Exting	Entering	Exiting	Capture*	Reduction	Entenny	Exting	Trips
182	TC	Multifamily Housing (Low-Rise) (220)	795	Dwelling Units	342	23%	77%	79	263	9%	2,5%	70	233	303
1 & 2 1 & 2	TC TC	Single-Family Detached Housing (210)	119 1272	Dwelling Units	90 1,224	25% 86%	75% 14%	23 1,053	68 171	9% 9%	2.5% 2.5%	20 934	60 152	80 1,086
1 & 2	TC	General Office Building (710) Shopping Center (820)	258.8	1,000 Sq. Ft. GFA 1,000 Sq. Ft. GLA	244	62%	38%	151	93	9%	2.5%	134	83	217
182	VC-C	Multifamily Housing (Low-Rise) (220)	498	Dwelling Units	220	23%	77%	51	169	5%	2.5%	47	157	204
182	VC-C	Single-Family Detached Housing (210)	78	Dwelling Units	62	25%	75%	16	47	5%	2,5%	15	44	59
1 & 2	VC-C	General Office Building (710)	31.9	1,000 Sq. Ft, GFA	58	86%	14%	50	8	5%	2,5%	46	7	53
1 & 2	VC-C	Shopping Center (820)	36.3	1,000 Sq. Ft. GLA	36	62%	38%	22	14	5%	2,5%	20	13	33
1	Other	Multifamily Housing (Low-Rise) (220)	573	Dwelling Units	252	23%	77%	58	194	0%	2,5%	57	189	246
1	Other	Single-Family Detached Housing (210)	119	Dwelling Units	90	25%	75%	23	68	0%	2.5%	22	66	88
2 & 3	VC-A	Multifamily Housing (Low-Rise) (220)	570	Dwelling Units	250	23%	77%	58	193	7%	2.5%	53	175	228
2 & 3	VC-A	Single-Family Detached Housing (210)	60	Dwelling Units	48	25%	75%	12	36	7%	2.5%	11	33	44
2 & 3	VC-A	General Office Building (710)	90.1	1,000 Sq. Fl. GFA	112	86%	14%	96	16	7%	2.5%	87	15	102
2 & 3	VC-A	Shopping Center (820)	45.4	1,000 Sq. Ft. GLA	44	62%	38%	27	17	7%	2.5%	24	15	39
2	Olher	Multifamily Housing (Low-Rise) (220)	486	Dwelling Units	216 268	23% 25%	77% 75%	50 67	166 201	0% 0%	2.5%	49 65	162 196	211 261
2	Olher VC-B	Single-Family Detached Housing (210) Multifamily Housing (Low-Rise) (220)	369 900	Dwelling Units Dwelling Units	386	23%	77%	89	297	2%	2.5% 2.5%	85	284	369
3	VC-B	Single-Family Detached Housing (210)	72	Dwelling Units	56	25%	75%	14	42	2%	2,5%	13	40	53
3	VC-B	Shopping Center (820)	40.5	1,000 Sq. Ft. GLA	40	62%	38%	25	15	2%	2,5%	24	14	38
3	Other	Multifamily Housing (Low-Rise) (220)	449	Dwelling Units	200	23%	77%	46	154	0%	2,5%	45	150	195
3	Olher	Single-Family Detached Housing (210)	43	Dwelling Units	36	25%	75%	9	27	0%	2.5%	9	26	35
4	Olher	Multifamily Housing (Low-Rise) (220)	1109	Dwelling Units	470	23%	77%	108	362	0%	2,5%	105	353	458
4	Olher	Single-Family Delached Housing (210)	90	Dwelling Units	70	25%	75%	18	53	0%	2.5%	18	52	70
		Project Total a.m. Peak Hour Trips			4,814			2,145	2,674			1,953	2,519	4,472
		k Hour	# of	Unit	Trip			Trips	Trips	Internal	Transit	Net Trips		Total p.m
Phase	Area	Land Use	Units	Type	Generation		Exiting 270/	Entering	Exting	Clipture*	Reduction		Exiting	Trips
1 & 2	TC	Multifamily Housing (Low-Rise) (220)	795	Dwelling Units	374	63%	37%	236	138	11%	2,5%	205	120 39	325 106
1 & 2	TC TC	Single-Family Detached Housing (210) General Office Building (710)	119 1272	Dwelling Units 1,000 Sq. Ft. GFA	122 1,276	63% 16%	37% 84%	77 204	45 1,072	11% 11%	2.5% 2.5%	67 177	930	1,107
1 & 2 1 & 2	TC	Shopping Center (820)	258.8	1,000 Sq. Ft. GFA	988	48%	52%	474	514	11%	2.5%	411	446	857
182	VC-C	Multifamily Housing (Low-Rise) (220)	498	Dwelling Units	248	63%	37%	156	92	13%	2.5%	132	78	210
1 & 2	VC-C	Single-Family Detached Housing (210)	78	Dwelling Units	82	63%	37%	52	30	13%	2.5%	44	25	69
1&2	VC-C	General Office Building (710)	31.9	1,000 Sq. Ft. GFA	40	16%	84%	6	34	13%	2.5%	5	29	34
1 & 2	VC-Ç	Shopping Center (820)	36.3	1,000 Sq. Ft. GLA	140	48%	52%	67	73	13%	2.5%	57	62	119
1	Olher	Multifamily Housing (Low-Rise) (220)	573	Dwelling Units	280	63%	37%	176	104	0%	2.5%	172	101	273
1	Olher	Single-Family Detached Housing (210)	119	Dwelling Units	122	63%	37%	77	45	0%	2.5%	75	44	119
2 & 3	VC-A	Multifamily Housing (Low-Rise) (220)	570	Dwelling Units	278	63%	37%	175	103	11%	2.5%	152	89	241
2 & 3	VC-A	Single-Family Detached Housing (210)	60	Dwelling Units	64	63%	37%	40	24	11%	2.5%	35	21	56
2 & 3	VC-A	General Office Building (710)	90.1	1,000 Sq. Ft. GFA	104	16%	84%	17	87	11%	2.5%	15	75	90
283	VC-A	Shopping Center (820)	45.4	1,000 Sq. Ft. GLA	174	48%	52%	84	90	11%	2.5%	73	78	151
2	Other	Multifamily Housing (Low-Rise) (220)	486	Dwelling Units	242	63%	37%	152	90	0%	2.5%	148	88	236
2	Other VC-P	Single-Family Detached Housing (210) Multifamily Housing (Low-Rise) (220)	369	Dwelling Units	356 418	63% 63%	37%	224 263	132 155	0% 7%	2.5%	218 238	129 141	347 379
3	VC-B VC-B	Single-Family Delached Housing (210)	900 72	Dwelling Units Dwelling Units	418 76	63%	37% 37%	263 48	28	7% 7%	2.5% 2.5%	238 44	25	69
3	VC-B	Shopping Center (820)	40.5	1,000 Sq. Ft. GLA	156	48%	52%	48 75	28 81	7% 7%	2.5%	68	73	141
3	Olher	Multifamily Housing (Low-Rise) (220)	449	Dwelling Units	226	63%	37%	142	84	0%	2.5%	138	82	220
3	Other	Single-Family Detached Housing (210)	43	Dwelling Units	46	63%	37%	29	17	0%	2.5%	28	17	45
4	Other	Multifamily Housing (Low-Rise) (220)	1109	Dwelling Units	504	63%	37%	318	186	0%	2.5%	310	181	491
4	Other	Single-Family Detached Housing (210)	90	Dwelling Units	92	63%	37%	58	34	0%	2.5%	57	33	90
		Project Total p.m. Peak Hour Trips			6,408			3,150	3,258			2,869	2,906	5,775

Land Use Code from the Institute of Transportation Engineers (ITE). <u>Van Gaazardor</u>a. 10th Edition, 2017. Internal capture rates based on the NCHRP 684 Internal Trip Capture Estimation Tool, which follows ITE methodologies for internal capture.
Transt reduction of 2.5% based on the transit ridership of comparable nearby Riverton City, based on 2017 American Community Survey estimates. Assumes bus-only Iransit and no light-rail.

	NCHRP 684 Internal Trip Capture Estimation Tool								
Project Name:	Olympia Hills	П	Organization:	Hales Engineering					
Project Location:	Salt Lake County		Performed By:	Josh Gibbons					
Scenario Description:	Town Center Area		Date:	10/22/2019					
Analysis Year:	2032		Checked By:	Scott Johnson					
Analysis Period:	AM Street Peak Hour		Date:	10/22/2019					

Land Use	Developme	ent Data (<i>For In</i>	formation Only)	Estimated Vehicle-Trips ³			
Land Use	ITE LUCs1	Quantity	Units	Total	Entering	Exiting	
Office	710	1,272	1,000 sq ft	1,224	1,053	171	
Retail	820	258.8	1,000 sq ft	244	151	93	
Restaurant				0			
Cinema/Entertainment				0			
Residential	210 & 220	914	dwelling units	433	102	331	
Hotel				0			
All Other Land Uses ²				0			
				1,901	1,306	595	

		Table 2-A:	Mode Split and Vehi	cle Occupancy Estimates				
Land Use		Entering Tri	ps		Exiting Trips			
	Veh. Occ.⁴	% Transit	% Non-Motorized	Veh. Occ.4	% Transit	% Non-Motorized		
Office	1.06	2.5%	0%	1.06	2.5%	0%		
Retail	1.17	2.5%	0%	1.17	2.5%	0%		
Restaurant								
Cinema/Entertainment								
Residential	1.13	2.5%	0%	1.13	2.5%	0%		
Hotel								
All Other Land Uses ²								

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)										
Ovinin (Franc)		Destination (To)								
Origin (From)	Office	Retail	Residential	Hotel						
Office										
Retail										
Restaurant										
Cinema/Entertainment										
Residential										
Hotel										

Table 4-A: Internal Person-Trip Origin-Destination Matrix*											
Origin (Fram)		Destination (To)									
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel					
Office		51	0	0	0	0					
Retail	32		0	0	2	0					
Restaurant	0	0		0	0	0					
Cinema/Entertainment	0	0	0		0	0					
Residential	7	4	0	0		0					
Hotel	0	0	0	0	0						

Table 5-A:	Computatio	ns Summary	
	Total	Entering	Exiting
All Person-Trips	2,072	1,408	664
Internal Capture Percentage	9%	7%	14%
External Vehicle-Trips ⁵	1,685	1,190	495
External Transit-Trips ⁶	47	33	14
External Non-Motorized Trips ⁶	0	0	0

Table 6-A: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	3%	28%						
Retail	31%	31%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	2%	3%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool								
Project Name:	Olympia Hills	Г	Organization:	Hales Engineering					
Project Location:	Salt Lake County		Performed By:	Josh Gibbons					
Scenario Description:	Town Center Area		Date:	10/22/2019					
Analysis Year:	2032		Checked By:	Scott Johnson					
Analysis Period:	PM Street Peak Hour		Date:	10/22/2019					

Land Use	Developme	ent Data (For In	formation Only)	Estimated Vehicle-Trips ³			
Land Use	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting	
Office	710	1,272	1,000 sq ft	1,276	204	1,072	
Retail	820	258.8	1,000 sq ft	988	474	514	
Restaurant				0			
Cinema/Entertainment				0			
Residential	210 & 220	914	dwelling units	496	313	183	
Hotel				0			
All Other Land Uses ²				0			
				2,760	991	1,769	

	Table 2-P: Mode Split and Vehicle Occupancy Estimates									
Land Use		Entering Tri	ps	П	Exiting Trips					
Land Ose	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.4	% Transit	% Non-Motorized			
Office	1.11	2.5%	0%		1.11	2.5%	0%			
Retail	1.21	2.5%	0%		1.21	2.5%	0%			
Restaurant										
Cinema/Entertainment										
Residential	1.15	2.5%	0%		1.15	2.5%	0%			
Hotel										
All Other Land Uses ²										

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)							
Origin (Fram)				Destination (To)			
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel	
Office		2000			2000		
Retail					2000		
Restaurant							
Cinema/Entertainment							
Residential		2000					
Hotel							

Table 4-P: Internal Person-Trip Origin-Destination Matrix*							
Origin (From)	Destination (To)						
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel	
Office		15	0	0	14	0	
Retail	12		0	0	102	0	
Restaurant	0	0		0	0	0	
Cinema/Entertainment	0	0	0		0	0	
Residential	8	18	0	0		0	
Hotel	0	0	0	0	0		

Table 5-P: Computations Summary							
	Total	Entering	Exiting				
All Person-Trips	3,182	1,160	2,022				
Internal Capture Percentage	11%	15%	8%				
External Vehicle-Trips ⁵	2,409	824	1,585				
External Transit-Trips ⁶	72	25	47				
External Non-Motorized Trips ⁶	0	0	0				

Table 6-P: Internal Trip Capture Percentages by Land Use							
Land Use	Entering Trips	Exiting Trips					
Office	9%	2%					
Retail	6%	18%					
Restaurant	N/A	N/A					
Cinema/Entertainment	N/A	N/A					
Residential	32%	12%					
Hotel	N/A	N/A					

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool						
Project Name:	Olympia Hills		Organization:	Hales Engineering			
Project Location:	Salt Lake County		Performed By:	Josh Gibbons			
Scenarlo Description:	Village Center A Area		Date:	10/22/2019			
Analysis Year:	2037	1	Checked By:	Scott Johnson			
Analysis Period:	AM Street Peak Hour		Date:	10/22/2019			

Land Use	Developme	ent Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	90.1	1,000 sq ft	112	96	16
Retail	820	45.4	1,000 sq ft	44	27	17
Restaurant				0		
Cinema/Entertainment				0		
Residential	210 & 220	630	dwelling units	299	70	229
Hotel				0		
All Other Land Uses ²				0		
				455	193	262

				le Occupancy Estimates		
Land Use		Entering Tri	ps		Exiting Trips	
Earld OSC	Veh. Occ.⁴	% Transit	% Non-Motorized	Veh. Occ.⁴	% Transit	% Non-Motorized
Office	1.06	2.5%	0%	1.06	2.5%	0%
Retail	1.17	2.5%	0%	1.17	2.5%	0%
Restaurant						
Cinema/Entertainment						
Residential	1.13	2.5%	0%	1.13	2.5%	0%
Hotel						
All Other Land Uses ²						

	Table	3-A: Average L	and Use Interchan	ge Distances (Feet Walking D	Distance)	
Origin (From)						
Oligili (Floili)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office						
Retail						
Restaurant						
Cinema/Entertainment						
Residential						
Hotel	100			The state of the s		

Table 4-A: Internal Person-Trip Origin-Destination Matrix*							
Origin (From)		Destination (To)					
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel	
Office		5	0	0	0	0	
Retail	4		0	0	2	0	
Restaurant	0	0		0	0	0	
Cinema/Entertainment	0	0	0		0	0	
Residential	3	3	0	0		0	
Hotel	0	0	0	0	0		

Table 5-A: Computations Summary						
	Total	Entering	Exiting			
All Person-Trips	509	213	296			
Internal Capture Percentage	7%	8%	6%			
External Vehicle-Trips ⁵	416	174	242			
External Transit-Trips ⁶	11	5	6			
External Non-Motorized Trips ⁶	0	0	0			

Table 6-A: Internal Trip Capture Percentages by Land Use						
Land Use	Entering Trips	Exiting Trip				
Office	7%	29%				
Retail	25%	30%				
Restaurant	N/A	N/A				
Cinema/Entertainment	N/A	N/A				
Residential	3%	2%				
Hotel	N/A	N/A				

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete,

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool						
Project Name:	Olympia Hills		Organization:	Hales Engineering			
Project Location:	Salt Lake County	1	Performed By:	Josh Gibbons			
Scenario Description:	Village Center A Area		Date:	10/22/2019			
Analysis Year:	2037		Checked By:	Scott Johnson			
Analysis Period:	PM Street Peak Hour	1	Date:	10/22/2019			

Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	90.1	1,000 sq ft	104	17	87
Retail	820	45.4	1,000 sq ft	174	84	90
Restaurant				0		
Cinema/Entertainment				0		
Residential	210 & 220	630	dwelling units	342	215	127
Hotel				0		
All Other Land Uses ²				0		
				620	316	304

		Table 2-P:	Mode Split and Veh	icle	Occupancy Estimates		
Land Use		Entering Trips			Exiting Trips		
Land Ose	Veh, Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized
Office	1.11	2.5%	0%		1.11	2.5%	0%
Retail	1.21	2.5%	0%		1.21	2.5%	0%
Restaurant							
Cinema/Entertainment							
Residential	1.15	2.5%	0%		1.15	2.5%	0%
Hotel							
All Other Land Uses ²							

	Table 3	-r. Average La	and ose interchan	ge Distances (Feet Walking	Distance)	
Origin (From)				Destination (To)		
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		1750			1750	
Retail					1750	
Restaurant						
Cinema/Entertainment						
Residential		1750	-			
Hotel						

Table 4-P: Internal Person-Trip Origin-Destination Matrix*											
Origin (Front)		Destination (To)									
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel					
Office		4	0	0	1	0					
Retail	2		0	0	20	0					
Restaurant	0	0		0	0	0					
Cinema/Entertainment	0	0	0		0	0					
Residential	6	5	0	0		0					
Hotel	0	0	0	0	0						

Table 5-P: Computations Summary									
Total Entering Exiting									
All Person-Trips	720	368	352						
Internal Capture Percentage	11%	10%	11%						
External Vehicle-Trips ⁵	542	276	266						
External Transit-Trips ⁶	15	8	7						
External Non-Motorized Trips ⁶	0	0	0						

Table 6-P: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	42%	5%						
Retail	9%	20%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	9%	8%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

Enter trips assuming no transit or non-motorized trips (as assumed in ITE Trip Generation Manual).

Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip	Cap	ture Estimation Tool	
Project Name:	Olympia Hills		Organization:	Hales Engineering
Project Location:	Salt Lake County		Performed By:	Josh Gibbons
Scenario Description:	Village Center B Area	7	Date:	10/22/2019
Analysis Year:	2037	1	Checked By:	Scott Johnson
Analysis Period:	AM Street Peak Hour		Date:	10/22/2019

Land Use	Developme	ent Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land OSe	ITE LUCs1	Quantity	Units	Total	Entering	Exiting
Office				0		
Retail	820	40.5	1,000 sq ft	40	25	15
Restaurant				0		
Cinema/Entertainment				0		
Residential	210 & 220	972	dwelling units	442	103	339
Hotel				0		
All Other Land Uses ²				0		
				482	128	354

		Table 2-A:	Mode Split and Veh	icle (Occupancy Estimates		
Land Use		Entering Tri	ps	П	Exiting Trips		
Land Ose	Veh. Occ,4	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized
Office							
Retail	1.17	2.5%	0%		1.17	2.5%	0%
Restaurant							
Cinema/Entertainment							
Residential	1.13	2.5%	0%		1.13	2.5%	0%
Hotel							
All Other Land Uses ²							

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)									
Origin (From)				Destination (To)					
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office									
Retail									
Restaurant									
Cinema/Entertainment									
Residential									
Hotel		0.00							

Table 4-A: Internal Person-Trip Origin-Destination Matrix*											
Origin (From)		Destination (To)									
Oligili (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel					
Office		0	0	0	0	0					
Retail	0		0	0	2	0					
Restaurant	0	0		0	0	0					
Cinema/Entertainment	0	0	0		0	0					
Residential	0	4	0	0		0					
Hotel	0	0	0	0	0						

Table 5-A: Computations Summary									
	Total	Entering	Exiting						
All Person-Trips	546	145	401						
Internal Capture Percentage	2%	4%	1%						
	100	T 440 I	0.14						
External Vehicle-Trips ⁵	460	119	341						
External Transit-Trips ⁶	13	4	9						
External Non-Motorized Trips ⁶	0	0	0						

Table 6-A: Internal Trip Capture Percentages by Land Use							
Land Use	Entering Trips	Exiting Trips					
Office	N/A	N/A					
Retail	14%	11%					
Restaurant	N/A	N/A					
Cinema/Entertainment	N/A	N/A					
Residential	2%	1%					
Hotel	N/A	N/A					

²Total estimate for all other land uses at mixed-use development site is not subject to Internal trip capture computations in this estimator,

Enter trips assuming no transit or non-motorized trips (as assumed in ITE Trip Generation Manual).

Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool								
Project Name:	Project Name: Olympia Hills Organization: Hales Engineering								
Project Location:	Salt Lake County		Performed By:	Josh Gibbons					
Scenario Description:	Village Center B Area		Date:	10/22/2019					
Analysis Year:	2037		Checked By:	Scott Johnson					
Analysis Period:	PM Street Peak Hour		Date:	10/22/2019					

Land Use	Developme	ent Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs1	Quantity	Units	Total Entering	Exiting	
Office				0		
Retail	820	40.5	1,000 sq ft	156	75	81
Restaurant			6	0		
Cinema/Entertainment				0		
Residential	210 & 220	972	dwelling units	494	311	183
Hotel				0		
All Other Land Uses ²				0		
				650	386	264

Table 2-P: Mode Split and Vehicle Occupancy Estimates									
l and the		Entering Tri	ps	П		Exiting Trips			
Land Use	Veh. Occ.4	% Transit	% Non-Motorized	Į	Veh. Occ.⁴	% Transit	% Non-Motorized		
Office									
Retail	1.21	2.5%	0%		1.21	2.5%	0%		
Restaurant									
Cinema/Entertainment									
Residential	1.15	2.5%	0%		1.15	2.5%	0%		
Hotel									
All Other Land Uses ²									

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)									
O-i-i- (F)				Destination (To)					
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office									
Retail					1500				
Restaurant						and the state of			
Cinema/Entertainment									
Residential		1500							
Hotel									

	Table 4-P: Internal Person-Trip Origin-Destination Matrix*								
Origin (Franc)		Destination (To)							
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		0	0	0	0	0			
Retail	0		0	0	20	0			
Restaurant	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0			
Residential	0	6	0	0		0			
Hotel	0	0	0	0	0				

Table 5-P: Computations Summary									
	Total	Entering	Exiting						
All Person-Trips	757	449	308						
Internal Capture Percentage	7%	6%	8%						
External Vehicle-Trips ⁵	592	356	236						
External Transit-Trips ⁶	17	10	7						
External Non-Motorized Trips ⁶	0	0	0						

Table 6-P: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	N/A	N/A						
Retail	7%	20%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	6%	3%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Olympia Hills		Organization:	Hales Engineering						
Project Location:	Salt Lake County		Performed By:	Josh Gibbons						
Scenario Description:	Village Center C Area		Date:	10/22/2019						
Analysis Year:	2032		Checked By:	Scott Johnson						
Analysis Period:	AM Street Peak Hour		Date:	10/22/2019						

Land Use	Developme	ent Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	31.9	1,000 sq ft	58	50	8
Retail	820	36.3	1,000 sq ft	36	22	14
Restaurant				0		
Cinema/Entertainment				0		
Residential	210 & 220	576	dwelling units	283	67	216
Hotel				0		
All Other Land Uses ²				0		
				377	139	238

		Table 2-A:	Mode Split and Vehic	le Occupancy Estimates			
Land Use		Entering Tri	ps		Exiting Trips		
Lanu Ose	Veh. Occ.4	% Transit	% Non-Motorized	Veh. Occ.4	% Transit	% Non-Motorized	
Office	1.06	2.5%	0%	1.06	2.5%	0%	
Retail	1.17	2.5%	0%	1.17	2.5%	0%	
Restaurant							
Cinema/Entertainment							
Residential	1.13	2.5%	0%	1.13	2.5%	0%	
Hotel							
All Other Land Uses ²	V———I						

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)									
Origin (From)		Destination (To)							
Origin (Florii)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office									
Retail									
Restaurant	A								
Cinema/Entertainment									
Residential						15			
Hotel									

	Table 4-A: Internal Person-Trip Origin-Destination Matrix*												
Origin (From)		Destination (To)											
Ongin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel							
Office		2	0	0	0	0							
Retail	2		0	0	2	0							
Restaurant	0	0		0	0	0							
Cinema/Entertainment	0	0	0		0	0							
Residential	2	2	0	0		0							
Hotel	0	0	0	0	0								

Table 5-A:	Table 5-A: Computations Summary									
	Total	Entering	Exiting							
All Person-Trips	423	155	268							
Internal Capture Percentage	5%	6%	4%							
		г								
External Vehicle-Trips ⁵	350	127	223							
External Transit-Trips ⁶	10	4	6							
External Non-Motorized Trips ⁶	0	0	0							

Table 6-A: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	8%	25%						
Retail	15%	25%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	3%	2%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Olympia Hills		Organization:	Hales Engineering						
Project Location:	Salt Lake County		Performed By:	Josh Gibbons						
Scenario Description:	Village Center C Area	1	Date:	10/22/2019						
Analysis Year:	2032		Checked By:	Scott Johnson						
Analysis Period:	PM Street Peak Hour		Date:	10/22/2019						

Land Use	Developme	ent Data (For In	formation Only)	Estimated Vehicle-Trips ³			
Land Ose	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting	
Office	710	31.9	1,000 sq ft	40	6	34	
Retail	820	36.3	1,000 sq ft	140	67	73	
Restaurant				0			
Cinema/Entertainment				0			
Residential	210 & 220	576	dwelling units	330	208	122	
Hotel				0			
All Other Land Uses ²				0			
				510	281	229	

	Table 2-P: Mode Split and Vehicle Occupancy Estimates											
Land Use		Entering Tri	ps		Exiting Trips							
Land Ose	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized					
Office	1.11	2.5%	0%		1.11	2.5%	0%					
Retail	1.21	2.5%	0%		1.21	2.5%	0%					
Restaurant												
Cinema/Entertainment												
Residential	1.15	2.5%	0%		1.15	2.5%	0%					
Hotel												
All Other Land Uses ²												

	Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)												
O-i-i- (F-o)		Destination (To)											
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel							
Office		1000			1000								
Retail					1000	7-1							
Restaurant													
Cinema/Entertainment													
Residential		1000											
Hotel													

	Table 4-P: Internal Person-Trip Origin-Destination Matrix*												
Origin (From)													
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel							
Office		5	0	0	1	0							
Retail	2		0	0	21	0							
Restaurant	0	0		0	0	0							
Cinema/Entertainment	0	0	0		0	0							
Residential	4	6	0	0		0							
Hotel	0	0	0	0	0								

Table 5-P:	Table 5-P: Computations Summary									
	Total	Entering	Exiting							
All Person-Trips	593	327	266							
Internal Capture Percentage	13%	12%	15%							
External Vehicle-Trìps ⁵	431	241	190							
External Transit-Trips ⁶	13	7	6							
External Non-Motorized Trips ⁶	n	0	0							

Table 6-P: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	86%	16%						
Retail	14%	26%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	9%	7%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE Trip Generation Manual).

Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

Person-Trips

*Indicates computation that has been rounded to the nearest whole number.



APPENDIX B

Updated Land Use Trip Generation

-				Sali Lake Trip G	a County eneration	Olympi - Phase	a Hills TI 4 (2042)	s						
Weeka	Ann	Late Live	e H Lank	100	100	-	4	Trin .	Total Events		Tennis.	Par Trans	Her France Colons	Treat Deby
182	TC	Muhlandy Housing Blow-Rises (220) Single-Family Delached Housing (210)	215	Dwelling Units Dwelling Units	2,104	50%	50%	1,052	1,052	67	2.5%	1,026	1,026	2,052
162	TC	General Office Building (710) Shopping Center (820)	1272 258.8	1,000 Sq. Fl. GFA 1,000 Sq. Fl. GLA	9,770	90%	50% 50%	6,253 4,885	6,253 4,885	63°	2.5%	8,097 4,763	6,097 4,763	12,194 9,526
182	VC-C	Multifamily Housing (Low-Ree) (220) Single-Family Detected Housing (210)	75 137	Dwaling Units Dwaling Units	528 1,390	90% 50%	50% 50%	264 695	264 695	0% 0%	2.5%	257 678	257 678	514 1,358
182	VC-C	General Office Building (710) Shooting Center (820)	31,9	1,000 Sq. Fl, GFA 1,000 Sq. Fl, GLA	1,372	50%	50% 50%	176 688	176 686	Oh.	2.5%	172 669	172 669	344
1.5	O044	Muhlandy Housing (Low-Kee) (200) Single-Family Detected Housing (212)	95 162	Owelling Units Owelling Units	678 1,622	90% 90%	50% 50%	339 811	339 811	6% 6%	2.5%	331 791	331 791	662 1,582
283	VCA VCA	Multilands Hayling (Low-Keel (221)) Single-Family Determed Housing (210)	88 148	Ownling Units Ownling Units	6 IO 1,492	50% 50%	50%	305 748	305 746	0% 0%	2.5%	297 727	297 727	594 1.454
283	YC4	General Office Building (PTD)	90.1	1,000 Sq. Ft. QFA	960	30%	50% 50%	480	480	4%	2.5%	468	466	936
2	Other	Shopping Comar (\$25) Multifamily Housing (Low-Ree) (\$22) Singsof amily Colambid Housing (\$25)	45,4	1,000 Sq. Fl. GLA Dwalling Units	1,714 838	50% 50%	60%	857 419	857 419	639	2.5%	636 409	836 409	1,672 816
3	VC-6	SAURIANS Houses Con-Keet (220)	200 133	Dwelling Units Dwelling Units	1,968 9 6 6	90%	50% 50%	984 483	984 483	0%	2.5%	959 471	959 471	1,916 942
3	VC#	Single-Family Detached Housing (215) Siveping Center (625)	227 40,5	Dwelling Units 1,000 Sq. Ft, GLA	2,212	50%	50%	1,108 765	1,106 765	en.	2.5%	1,078 746	1,078 746	2,156 1,492
3	0741 0741	Muhllandy Housing (Low-Mary) (220) English andy Delastrad Housing (200)	67 115	Dwelling Units Dwelling Units	466 1,184	50% 50%	50% 50%	233 592	233 592	270	2.5%	227 577	227 577	454 1,154
4	Other	Multipliamity Howards (Conc. Prints) (220)	165 276	Dwelling Units Dwelling Units	1,208	50% 50%	50%	604 1,323	804 1,323	6%	2.5%	589 1,290	589 1,290	1,178
182	ŤC	Single of arridy Detactions Plansing (2715) Multilately Plansing (Manthau) (221)	470	Dwelling Units	2,560	50%	50%	1,280	1,280	0% 0%	2.5%	1,246	1,248	2,498
182	TC	Sensor Adult Housing-Detailted (251) Sensor Adult Housing-Adapted (252)	43	Dwaling Units Dwaling Units	366 148	50%	50% 50%	183 74	183 74	674	2.5%	176 72	178 72	358 144
182	AC-C	Multimity Housing (Mid-Mar) (221) Service Alfult Housing-Clatected (251)	300 38	Dwelling Units Dwelling Units	1,634	50%	50% 50%	817 121	817 121	9% 9%	2.5%	797 118	797	1,594
1 & 2	VC-C Other	Sense All Meaning Atlantical (252) Multimity Housing (Markins) (221)	26 357	Dwelling Units Dwelling Units	80 1,944	50%	50% 50%	40 972	40 972	0% 9%	2.5%	39 946	39 948	78 1,698
1	Other	Senor Adult Housey-Ortamed (251) Senor Adult Housey-Married (252)	46 32	Owelling Units Owelling Units	286 104	90% 90%	50% 50%	143 52	143 52	0% 0%	2.5%	139 51	139 51	278 102
283 283	VC-A	Multilands Housing (McGiffred (221)	325	Ownling Units	1,770	50%	50% 50%	885	885	0%	2.5%	863	863	1,728
283	VC-A	Senior Adult Housing-Detached (251) Senior Adult Housing-Milested (252)	42 29	Owelling Units Owelling Units	92	50%	50%	132 46	132 46	0%	2.5%	129 45	129 45	258 90
2	Other	Multilandy Housing (Middless (221) Service Adult Housing Chilarhook (251)	442 57	Dwelling Units Dwelling Units	2,408 344	50%	50% 50%	1,204	1,204	976	2.5%	1,174 168	1,174	2,348 336
2	Other VC-8	Sener AQA Housey-Maches (252) Multismity Housing (Modilles) (221)	40 502	Drealing Units Dwalling Units	136 2,738	50% 50%	50% 50%	68 1,360	68 1,368	0% 0%	2,5%	66 1,334	88 1,334	132 2,668
3	VC-B	Senor Albit Heuring-Driteched (251)	65 45	Dwelling Units Dwelling Units	388 196	50%	50%	193	193	Ch.	2.5%	188	188	378 152
3	Other	Multifamily Housing (Missilian) (221)	254	Dwelling Units	1,384	50%	50%	692	692	0%	2.5%	575	675	1,350
3	Other	Server Adult Housing-Distacted (251) Server Adult Housing-Ritached (252)	33 23	Dwelling Units Dwelling Units	214 68	50%	50% 50%	107 34	107 34	6% 6%	2,5% 2.5%	104 33	104 33	208 86
4	Other	Guttfamily Housing (Mis-Rise) (221) Senior Adult Housing-Distarted (251)	819 83	Dwelling Units Dwelling Units	3,372 478	50% 50%	50% 50%	1,688 239	1,686 239	0%	2.5%	1,644 233	1,644	3,288 468
4	Other	Senor Adult Housey-Harmed (252) Project Tobal Dolly Trigal	58	Dwaling Units	70.794	50%	50%	100 25,397	100	th	2.5%	98 34.320	98 34.329	198 68,640
Moint	g Pe	nk Hour	Total Lines		tu.			100	10	Second Carried	Janes .	NATION IN	Heat Sales	falation.
182	fig fic	Multitarily Houses (Low-Res) (220) Single-Family Detection Houses (210)	215	OwnEng Units Dwaling Units	60 158	23% 25%	77% 75%	14	119	15	2.5%	12	106	53 141
182	TC	General Office Building (713)	1272	1,000 Sq. Ft. GFA	1,224	86% 62%	14%	1,7553	171	\$5.	2.5%	934	152	1,088
1 & 2	VC-C	Shopping Corner (630) Multilandy Housing (Low-Rest) (2015)	258.8 75	1,000 Sq. Fl. GLA Dwelling Units	244 38	2314	77%	151	93 29	5%	2.5%	134	83 27	217 35
182 152	VC-C	Single-Family Delected Housing (210): General Office Building (210):	137 31,9	Dwaling Unite 1,000 Sq. Ft, GFA	104 58	25% 86%	20%	24 50	78 8	5%	2.5%	24 48	72 7	96 53
182	VC-C Other	Shopping Center (820)	36.3 95	1,000 Sq. Fl. GLA Dwelling Units	36 46	62% 23%	1874 77%	22	14 35	5%	2.5%	20 11	13	33 45
243	Other VC-A	Single-Family Delethout Housing (210)	162 570	Dwalling Units	120 250	25%	79% 77%	30 64	90 193	55	2,5%	29 54	88 179	117
283	VC-A	Mullianity Housing (Low-Rest) (222) Single-Family Detailshed Housing (210)	60	Dwalling Units Dwalling Units	48	25%	12%	12	36	55	2.5%	11	11	44
2 & 3 2 & 3	VC-A	General (Affice Budsing (712) Shopping Center (820)	90,1 45,4	1,000 Sq. Fl. GFA 1,000 Sq. Fl, GLA	112	86% 62%	38%	27	16 17	5%	2.5%	89 25	15 18	104 41
2 2	Other	Multilandy Housing (Low-Rine) (221) Single-Family Outsided Housing (215)	118	Dwalling Units Dwalling Units	56 148	25%	77%	13	43 111	DN:	2.5%	13	42 108	55 144
3	VC-B	Multiplier of the same of the control of the contro	133 227	Dwalling Units Dwalling Units	64 166	23% 25%	77%	15	49 125	1% 22s	2.5%	14	46 118	60 158
3	VC-B Other	Straying Caries (\$25)	40,5 67	1,000 Sq. Fl. GLA	40	62%	38°C	25	15 26	3%	2.5%	24	14	38 33
3	Other	Single-Family Detailment Housing (210)	115	Dwelling Units Dwelling Units	88	25%	22%	22	56	th.	2,5%	21	25 64	85
1	Other	Multilandy Housing (Low-Rest) (220) Single-Family Directors Housing (210)	165 278	Dwaling Units Owaling Units	79 202	23% 25%	75%	**	50 152	en.	2.5%	18 50	59 148	77 198
182	TC	Multisorly Housing (Mid-River) (221) Senior Adult Housing-Delected (251)	470 51	Dwelling Units Owelling Units	30	26% 33%	87%	10	126 20	5%	2.5%	39 B	112 18	151 27
142	TC VC-C	Service Acute Province Assessed (252) Multilandy Housing (RAS-Place) (221)	43 300	Ovelling Units Ovelling Units	10	35% 26%	65% P4%	4	7 80	15	2.5%	4 25	8 74	100
182	VC-C	Server Adult Houseng-Detection (251) Server Adult Houseng-Reserved (252)	38 26	Owelling Units Dwelling Units	20 8	33%	65%	7	13	55	2.5%	6 2	12	18
1	Other	Multilately Housing (Mid-Hee) (221)	357	Dwelling Units	130	26%	24%	34	96	Q%	2.5%	33	94	127
1	Other	Server Adult Housing-Distanced (251) Server Adult Housing-Educated (252)	46 32	Dwalling Units Dwalling Units	24 8	33% 35%	65%	1	18	0%	2.5%	3	16 5	24
2 & 3 2 & 3	VC-A	Multilandy Husing (McGRee) (221) Senor Adult Housing-Determed (251)	325 42	Dwelling Units Dwelling Units	118	26% 33%	52%	21	87 15	5%	2.5%	29 8	91 14	110 20
283	VC-A Other	Servor Adult Howard-Abached (202) Multilanily Howard (Modified) (221)	29 442	Dwalling Units Dwalling Units	6 160	35% 26%	20%	40	118	5%	2.5%	2 41	115	6 158
2 2	Other	Server Adult House of Catalogue (251) Server Adult House of Allested (251)	57 40	Dwelling Units Dwelling Units	28 8	35%	67%	1	19 5	en.	2.5%	9	19	28 6
3	VC-B	Multimay Housing (Mid-Place) (221)	502	Dwelling Unite	182 30	26% 33%	24% 62%	47 10	135	In In	2.5%	44		172
													128	
3	VC-B	Server Adult Housing-Deserted (251) Server Adult Housing-Haarted (252)	65 45	Dwelling Units Dwelling Units	10	15%	45%	. A.	7	2%	2.5%	9	19 7	28 11
3	VC-B VG-B Other	Senior Adult Housing-Attented (\$52) Multismit, Housing (Mid-Root) (\$21) Senior Adult Housing-Detached (\$21)	45 254 33	Dwaling Units Dwaling Units Dwaling Units	92 18	35% 28% 33%	95% 24% 87%	*	68 12	0% 0%	2.5%	4 23 6		28 11 69 18
3	VC-B VG-B Other Other Other	Sinner Adult Housing-Atlantind (452) Martining, Housing (Mid-Bress (221) Sinner Adult Housing-Disserted (251) Sinner Adult Housing-Atlantind (453) Martining Housing (Mid-Bress (231)	45 254 33 23 619	Owelling Units Dwelling Units Owelling Units Owelling Units Owelling Units	92 18 6 224	15% 26% 33% 35% 26%	85% 34% 87% 85% 74%	**	68 12 4 186	0% 0% 0%	2.5% 2.5% 2.5% 2.5%	4 23 6 2 57	19 7 56 12 4 162	28 11 89 18 6 219
3	VC-B VC-B Other Other Other	Senior Adult Housey-Monthel (192) Multilandy Housey-Modeley (201) Senior Adult Housey-Modeley (201) Senior Adult Housey-Modeley (201) Multilandy Housey-Modeley (201) Senior Adult Housey-Modeley (201) Senior Adult Housey-Modeley (201)	45 254 33 23	Dwaling Units Dwaling Units Dwaling Units Owelling Units Owelling Units Owelling Units Owelling Units	92 18 6 224 36	15% 28% 33% 35%	85% 34% 85% 85%		68 12 4 186 24	0% 0%	2.5% 2.5% 2.5%	4 23 6 2 57 12 4	19 7 56 12 4	28 11 89 18 6
3 3 4 4 4	VC-B VG-B Other Other Other Other Other	Sinner Adult Housing-Atlantind (452) Martining, Housing (Mid-Bress (221) Sinner Adult Housing-Delasteds (251) Sinner Adult Housing-Atlantind (453) Martining-Housing (Mid-Bress (231)	45 254 33 23 619 83	Owelling Units Dwelling Units Owelling Units Owelling Units Owelling Units	92 18 6 224 36	35% 33% 35% 26% 33%	855 875 855 855 875	**	68 12 4 186	50. 50. 50. 50.	2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57	19 7 56 12 4 162 23	28 11 89 18 6 219 35
3 3 4 4	VC-B VG-B Other Other Other Other Other	Service Adult Housesp-Statemed (1935) Markhamby Housesp-Statemed (1935) Service Adult Housesp-Statemed (2615) Service Adult Housesp-Statemed (2615) Service Adult Housesp-Statemed (2615) Service Adult Housesp-Statemed (2615) Service Adult Housesp-Statemed (1935) Project Trail a.m. From House Trape 104 HOUSE Service Adult Housesp-Statemed (1935) Project Trail a.m. From House Trape 104 HOUSE Service Adult Housesp-Statemed (1935) Project Trail a.m. From House Trape 105 HOUSE Service Adult Housesp-Statemed (1935) Project Trail a.m. From House House Trape 105 HOUSE Service Adult Housesperies (1935) Project Trail A.m. From House House Trape 105 HOUSE Service Adult House Trape	45 254 33 23 619 83	Owelling Units	92 18 6 224 36	35% 33% 35% 26% 33%	855 875 855 855 875		68 12 4 186 24 2,551	50. 50. 50. 50.	2,5% 2,5% 2,5% 2,5% 2,5% 2,5% 2,5%	4 23 6 2 57 12 4	19 7 56 12 4 162 23 8	28 11 89 18 8 219 35 12 4,533
3 3 4 4 4 4 4 152	VC-8 VC-8 Other Other Other Other Other Other	Senior Adult Housespellerstered (1935) Michiemy Housespellerstered (1935) Senior Adult Housespellerstered (2915) Senior Adult Housespellerstered (2915) Michiemy Housespellerstered (2915) Senior Adult Housespellers	45 254 33 23 619 83 56	Dwelling Units Dwelling Units Dwelling Units Dwelling Units Owelling Units Dwelling Units Dwelling Units Dwelling Units Dwelling Units Dwelling Units	92 18 6 224 36 11 4.871	15% 26% 33% 35% 26% 33% 35%	85 E 65 E	22311 47 134	68 12 4 186 24 2.133	25 65 65 65 65 65 65 65 65 65 65 65 65	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 2,937	19 7 56 12 4 162 23 8	28 11 89 18 6 219 35 12 4,531
3 3 4 4 4 4 4 162 182 182	VC-B VC-B Other Other Other Other TC TC	Sminn And Woursey-Brasted (1932) Makhamy shroung-Brasted (1932) Sminn And Huang-Brissler (2513) Sminn And Huang-Brissler (2514) Sminn And Huang-Brissler (2514) Makhamy Sminney (Marikhan) (2213) Sminn And Huang-Brissler (1932)	45 254 33 23 619 83 56 425 215 1272 258.8	Dwelling Units Look Sq. Fl. GFA Look Sq. Fl. GFA	92 18 6 224 36 11 212 1,276 988	15% 28% 33% 35% 26% 13% 35%	85% 85% 85% 85% 85% 85% 85% 85% 85% 85%	2,231 134 204 474	68 12 4 186 24 2.131 78 1,072 514	25. 65. 65. 66. 66. 66. 66. 66. 66. 66. 6	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 2,017	19 7 56 12 4 162 23 8 2 11 68 930 448	28 11 89 18 6 219 35 12 4,53 14 1,53 15 16 17 18 18 19 18 18 18 18 18 18 18 18 18 18 18 18 18
3 3 4 4 4 4 152 152 152 152 152 152	VC-B VG-B Other	Serion André Houseup-descried (\$125) Mackhami, Houseup-descried (\$125) Serion André House (\$25) Serion André Houseup-Disserber (\$25) Serion André Houseup-Disserber (\$15) Maldaminy Houseup-Disserber (\$15) Maldami	45 254 33 23 619 82 56 215 1272 258.8	Deeling Units	92 18 6 224 36 11 212 1,276 988 45 138	15% 28% 33% 35% 26% 35% 63% 63% 63%	85% 16% 15% 15% 15% 15% 15% 15% 15% 15% 15% 15	2231 134 204 474 29 87	68 12 4 186 24 2 2 187 78 1.072 514 17 61	85 65 65 65 65 65 65 65 65 65 65 65 65 65	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 2.017 116 177 411 25 74	19 7 56 12 4 162 23 6 8 930 448 14 43	28 11 89 18 8 219 35 12 4,531 44 1,107 857 39 117
3 3 4 4 4 4 4 152 182 182 182	VC-B VC-B Other Other Other Other Other TC TC TC VC-C VC-C VC-C VC-C	Senior Acida Houseage destined (\$125) Makitaming Houseage (\$125) Senior Acid Houseage (\$125) Senior Houseage (\$125) Seni	45 254 33 619 83 56 125 1272 258.8 75 137 11.9 36.3	Deathing Units 1,000 Sg. Fr. GLA	92 18 6 224 36 11 212 1,276 988 45 138 40 (40	15% 28% 33% 35% 26% 33% 35% 63% 63% 63% 63% 64%	95% 95% 95% 95% 95% 95% 95% 95% 95% 95%	2711 134 204 474 29 87 8 6	68 12 4 186 24 8 2 151 78 1.072 514 17 51 34 73	85 85 85 85 85 85 85 85 85 85 85 85 85 8	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 8 2 57 12 4 2.917 116 177 411 25 74 5 57	19 7 56 12 4 152 23 8 141 68 930 446 443 29 62	28 11 89 18 6 219 35 12 4,531 14 184 1,107 857 39 117 34 119
3 3 4 4 4 4 4 1 5 2 1 8 2 1 1 1 1	VC-B VC-B Other	Semin Adul Houseng-bitterfed (\$120) Makitamin Houseng-bitterfed (\$121) Semin Adul Houseng-bitterfed (\$121) Semin Adul Houseng-bitterfed (\$121) Makitamin Houseng-bitterfed (\$121) Makitamin Houseng-bitterfed (\$121) Makitamin Houseng-bitterfed (\$121) Seminan Adul Houseng-bitterfed (\$121) Makitamin Houseng-bitterfed (\$121) Seminan Adul Houseng-bitterfed (\$121) Makitamin Houseng-bitterfed (\$121) Makitamin Houseng-bitterfed (\$121) Makitamin Houseng-bitterfed (\$122) Seminan Houseng-bitterfed (\$122) Makitamin Hous	45 254 33 23 619 83 56 215 1272 258.8 75 137 11.9	Desaling Units Desaling Units Desaling Units Desaling Units Overlang Units Overlang Units Desaling Units	92 18 6 224 36 11 212 1,276 988 45 138 40 140 58	15% 28% 33% 35% 26% 13% 35% 63% 16% 48% 63% 16% 48% 63%	95% 95% 95% 95% 95% 95% 95% 95% 95% 95%	134 204 474 29 87 8 67 37 102	68 12 4 186 24 2 17 78 1.072 514 17 51 34 73 21 80	11% 11% 11% 11% 12% 13% 13% 13% 13%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 2.017 116 177 411 25 74 5	19 7 56 12 4 162 23 6 8 930 446 14 43 29	28 11 89 18 6 219 35 12 1,531 44 1,107 857 39 117
3 3 4 4 4 4 4 152 152 152 152 152 152 152 152 152 152	VC-B VC-B Other Other Other Other Other TC TC TC VC-C VC-C VC-C Other	Benin Adul Housey Jelimine (1923) Makitaniy Housey Jelimine (2021) Samira Adul Housey Schiller (2021) Samira Adul Housey Schiller (2021) Samira Adul Housey Schiller (2021) Samira Adul Housey Glüsserker (2021) Samira Glüsser (2021) Samira Glüsserker (2021) Samira Glüsserker (2021)	45 254 33 23 619 81 56 215 1272 258.8 75 137 11.9 36.0 95	Desahing Units Desahing Units Desahing Units Desahing Units Overlang Units Overlang Units Desahing Units	92 18 6 224 36 11 212 1,276 988 45 138 40 140 58	15% 28% 33% 35% 26% 31% 35% 63% 16% 63% 63% 63%	85% 55% 55% 55% 55% 55% 55% 55% 55% 55%	134 2231 134 204 474 29 87 8 6 67 37	68 12 4 186 24 186 24 17 78 1,072 514 17 51 34 73 21 60 19	11% 11% 11% 11% 11% 13% 13% 13% 13%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 2.017 116 117 25 74 5 5 7 36 99 29	19 7 56 12 4 162 23 8 14 43 29 52 20 59	28 11 89 18 6 219 35 12 4,538 44 1,507 39 119 36 155 36 155 46
3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-B VC-C VC-C VC-C VC-C VC-C VC-C VC-C	Serion André Houseup de lamine (1923) Mail Amerika (1924) Mail Amerika (1924) Serion André House (2721) Serion André Houseup (1924)	45 234 33 619 83 83 56 215 1272 258.8 75 137 11.9 36.0 95 162 86 148 90.1	Deading Uhia Deading Uhia Deading Uhia Deading Uhia Overlang Uhia Deading Uhia	92 18 6 224 36 11 4 17 212 1,276 988 46 149 58 162 52 150 104	15% 26% 33% 35% 26% 35% 35% 48% 63% 63% 48% 63% 48% 63% 48% 63% 16%	374 524 374 524 374 524 374 524 374 524 374 374 644	47 134 204 474 29 87 6 67 37 102 33 95	68 12 4 186 24 78 1,072 514 17 51 34 73 21 90 19	11% 11% 11% 11% 12% 12% 12% 12% 12% 10% 10% 10%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 2.017 11 25 74 11 25 77 36 99 29 83 15	19 7 56 12 4 162 23 8 8 14 48 14 43 29 62 20 59 17 49 76	28 11 89 18 8 219 35 12 4,538 14 104 1,107 39 117 39 119 56 158 46 119 57 39 119 56 119 57 39 119 58 119 58 119 58 119 58 119 58 119 58 119 58 119 58 119 58 119 58 119 58 58 58 58 58 58 58 58 58 58 58 58 58
3 3 4 4 4 4 152 152 162 162 162 162 162 162 162 162 162 16	VC-8 VC-8 Other VC-C VC-C VC-C VC-C VC-C VC-C VC-C VC-	Semin And Houses planted (\$12) Makitami, Houses planted (\$12) Semin And Houses planted (\$12) Semin And Houses planted (\$13) Semin And Houses planted (\$10) Makitami Houses (\$10) Semin And Houses (\$10)	45 254 31 23 619 83 56 215 1272 258.8 75 137 11.9 36.3 95 162 89.1 45.4	Deeding Uhita 1,000 St. Pt. GFA. 1,000 St. Pt. GFA. Deeding Uhita Deeding Uhita 1,000 St. Pt. GFA. Deeding Uhita D	92 18 6 224 36 212 1,276 988 46 138 40 140 158 162 52 150 104 174 86	15% 26% 33% 35% 26% 35% 35% 63% 48% 63% 48% 63% 48% 63% 48% 63% 48% 63% 48% 63% 63% 63% 63% 63% 63% 63% 63% 63% 63	374 844 524 3174 844 524 3174 3174 3174 3174 3174 3174 3174	134 204 474 29 87 6 67 37 102 33 95 17 84	68 12 4 186 24 2 187 78 1,072 514 17 34 73 21 80 19 66 87 90 25	11% 11% 11% 13% 13% 13% 10% 10% 10% 10% 10% 10% 10% 10%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 2.011 41 116 177 18 5 57 18 99 29 15 74 42	19 7 66 12 4 162 23 8 8 930 446 14 43 29 62 20 59 17 49 76 79 76	28 11 89 16 8 8 219 35 35 4,531 44 1,107 857 39 117 39 119 46 156 156 156 156 157 157 157 157 157 157 157 157 157 157
3 3 4 4 4 4 152 152 152 152 152 152 152 152 152 152	VC-8 VC-8 VC-9 Other Other Other Other TC TC TC TC VC-C VC-C VC-C VC-C VC-C VC	Benin Ander Housen gehörn für (1922) Mackbern, Housen gehörn für (1922) Benin Ander House (1921) Benin Ander Housen gehörn für (1922) Benin Ander Housen gehörn für (1923) Benin Ander Housen Housen (1923) Benin Ander Housen (1923) Benin Benin Benin Benin Benin Benin Benin Benin B	45 254 33 23 619 83 56 215 1272 238.6 75 137 11.9 36.3 95 162 86 148 80.1 45.4 116 200 133	Deephing Uhris	92 18 6 224 36 11 212 1,276 288 45 138 40 140 150 150 104 174 186 198	15% 26% 25% 35% 35% 35% 35% 16% 48% 63% 63% 63% 63% 63% 63% 63% 63% 63% 63	55% 55% 55% 55% 55% 57% 57% 57% 57% 57%	17 134 204 474 29 87 8 67 37 102 33 95 17 8 43 128 49	68 12 4 186 24 2 180 24 2 107 78 107 2 514 17 73 21 80 90 25 73 90 25 73 29	11% 11% 11% 12% 13% 13% 13% 10% 10% 10% 10% 10% 10% 10% 10% 10% 10	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 4 2.017 116 117 25 74 11 15 5 57 12 42 122 45	19 7 66 12 4 162 23 8 8 330 448 14 43 29 62 20 59 17 49 76 79 24 71 27	28 11 89 16 8 8 219 35 35 4,531 14 14 1,107 857 39 117 36 156 158 46 122 91 159 150 150 150 150 150 150 150 150 150 150
3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-8 VC-8 VC-8 VC-8 VC-8 VC-8 VC-8 VC-8	Senior Acida Historia philosophi (1922) Makilamin, Havang Senioria (1923) Makilamin, Havang Senioria (1924) Makilamin, Havang Chemiton (1923) Makilamin, Havang Chemiton (1924)	45 254 33 23 619 83 56 215 1272 238.8 75 137 21.9 36.3 95 162 89.1 118 200 133 227 40.5	Designing Uhrist Design	92 19 6 224 36 11 212 1,275 988 45 138 40 140 150 162 52 150 104 174 86 198 76 224 156	15% 28% 33% 35% 26% 31% 35% 63% 16% 48% 63% 63% 63% 63% 63% 63% 63% 63% 63% 64%	85% 85% 85% 85% 85% 85% 85% 85% 85% 85%	134 204 474 29 67 67 37 102 33 95 17 84 43 125 49 141 75	68 12 4 186 24 4 78 1.072 514 17 61 34 73 21 60 19 66 87 70 25 73 29 88	11% 11% 11% 12% 13% 13% 13% 10% 10% 10% 10% 10% 10% 10% 10% 10% 10	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 1 116 177 411 25 74 5 57 35 99 29 83 15 74 42 122	19 7 66 12 4 162 23 8 8 930 448 14 43 29 20 59 76 79 24 76	28 11 89 18 6 219 35 12 4,531 14 1,107 39 17 34 119 56 122 91 155 165 165 165 165 165 165 165 165 16
3 3 4 4 4 4 162 162 162 162 162 162 162 162 162 162	VC-8 VC-9 VC-9 VC-9 VC-9 VC-9 VC-9 VC-9 VC-9	Semin Audit Houseng-desirated (\$125) Makitamin Shangang-desirated (\$125) Samin Audit Houseng-Dissipated (\$12	45 254 33 23 619 83 56 127 235.8 1272 258.8 137 11.9 95 162 26 86 145.4 118 200 133 227 40.5 57	Dealing Units	92 19 6 224 36 11 212 1,276 988 46 138 40 140 58 162 152 150 104 174 86 198 78 224 156	15% 28% 35% 35% 26% 13% 35% 63% 63% 63% 48% 53% 63% 63% 63% 63% 63% 63% 63% 63% 63% 6	374 374 374 374 374 374 374 374 374 374	134 204 474 29 87 8 67 37 102 33 95 17 84 43 125 49 141 75 28	68 12 4 188 24 4 78 1,072 514 17 51 34 73 21 60 19 66 87 90 25 73 29 83 81 16	11% 11% 11% 12% 12% 12% 12% 12% 10% 10% 10% 10% 10% 10% 10% 10% 10% 10	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 77 12 4 1 116 177 4 11 25 7 7 35 99 99 99 83 15 74 2 122 45 129 69 25	19 7 7 66 12 4 162 23 5 8 930 446 14 43 29 52 20 59 17 49 76 79 24 71 27 76 76 77	28 111 89 18 6 219 35 12 4,531 14 1,532 11 107 857 39 117 24 191 195 164 164 175 185 185 185 185 185 185 185 185 185 18
3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-8 VC-C Other VC-C VC-C VC-C VC-C VC-C VC-C VC-C VC-	Beninn Ander Housensy elemined (1922) Makillamin, Hausen Jahn Baus (2021) Makillamin, Hausen Jahn Baus (2021) Makillamin, Hausen Jahn Baus (2023) Makillamin, Hausen Jahn Bausen Jahn Bausen Ander Hausen Jahn Bausen Jahren	45 254 33 33 23 36 19 619 619 619 619 619 619 619 619 619	Demandry Units.	92 19 6 224 36 11 212 1,276 988 45 138 40 140 58 162 52 150 104 178 86 198 76 224 186 224 189 98	13% 28% 33% 28% 33% 26% 33% 26% 33% 26% 33% 26% 35% 26% 35% 26% 26% 26% 26% 26% 26% 26% 26% 26% 26	274 274 274 274 274 274 274 274 274 274	47 47 47 47 47 47 47 47 47 47 47 47 47 4	68 12 4 186 24 ***********************************	11% 11% 11% 11% 12% 13% 13% 10% 10% 10% 10% 6% 6% 6% 6%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 77 12 4 4 2.011 25 5 5 5 7 4 2 122 4 5 123 69 25 7 2 5 8 6 7 8	19 7 56 12 4 162 23 8 8 930 446 14 43 29 52 20 59 76 76 76 77 76 74 77 76 74 77 76 74 77 76 77 76 77 76 77 77 77 77 77 77 77	28 11 89 16 6 17 219 35 219 35 24,531
3 3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-8 VC-0 Other TG	Senior Acida Houseng-bissioned (\$125) Makilaming Houseng-bissioned (\$125) Senior Acid Houseng-bissioned (\$125) Senior Bissioned (\$1	455 254 233 233 233 851 851 852 55 852 1272 228,8 851 137 11.9 10.5 10.5 10.5 10.5 10.5 10.5 10.5 10.5	Demining Units Design Units Des	92 19 6 224 36 11 212 1,276 958 46 138 162 52 150 104 174 86 198 78 224 1198 78 224 1198 224 1198 224 1198 226 227 227 228 238 248 249 249 249 249 249 249 249 249 249 249	35% 28% 33% 26% 33% 35% 35% 46% 46% 46% 46% 46% 46% 46% 46% 46% 46	52% 52% 52% 52% 52% 52% 52% 52% 52% 52%	2.231 134 2.231 134 204 47a 29 37 102 33 38 517 102 33 317 175 59 110 127	68 12 4 166 24 1 1,072 514 17 78 21 19 60 19 65 67 90 52 52 90 83 16 44 43 53 53 53 53 54 54 54 54 54 54 54 54 54 54 54 54 54	11% 11% 11% 11% 12% 13% 05, 10% 10% 10% 10% 10% 10% 10% 10% 10% 10%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 77 12 4 4 116 177 141 25 57 74 74 15 15 17 14 12 12 12 12 12 12 12 13 15 16 17 17 17 18 18 19 19 19 19 19 19 19 19 19 19 19 19 19	19 7 56 12 4 152 23 8 152 24 152 24 152 25 152 152 152 152 152 152 152 152	28 11 89 11
3 3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-8 VC-0 Other TG TG TG TG TG TG VC-C VC-C Other VC-A VC-A VC-A VC-A VC-A VC-A VC-A VC-A	Benin Ande Houseng-bissined (\$125) Makitamin, Houseng-bissined (\$121) Samus Ande Houseng-Bissined (\$121) Samus Anderson (\$122) Samus Anderson (\$122	45 254 254 255 256 257 257 257 257 257 257 257 257 257 257	Demining Units Demini	92 16 6 6 224 36 18 6 18 78 18 18 18 18 18 18 18 18 18 18 18 18 18	33% 33% 33% 33% 33% 33% 33% 33% 33% 33%	37% 84% 84% 85% 37% 37% 37% 37% 37% 37% 37% 37% 37% 37	2.231 134 2.231 134 294 474 29 37 102 33 35 17 17 17 17 29 110 12 12 13 14 15 16 17 17 18 18 18 18 18 18 18 18	58 12 4 186 24 2 4 186 24 186	11% 11% 11% 11% 12% 13% 10% 10% 10% 10% 10% 10% 10% 10% 10% 11% 11	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 57 12 4 4 2.837 41 116 74 41 25 74 59 59 59 59 69 15 17 42 42 43 43 44 41 41 41 41 41 41 42 43 43 44 44 45 46 47 47 47 48 48 48 48 48 48 48 48 48 48	19 7 7 59 12 4 162 23 8 2 44 1 162 23 6 6 1 162 20 7 16 16 16 17 17 17 17 17 17 17 17 17 17 17 17 17	28 11 80 11
3 3 3 4 4 4 4 162 162 162 162 162 162 162 162 162 162	VC-B VC-C VC-C VC-C VC-C VC-C VC-C VC-C	Beninn Andre Housen gehörn fed (\$12) Makillenin, Housen gehörn fed (\$12) Beninn Andre Housen (\$1	45 254 33 3 23 24 25 25 25 25 25 25 25 25 25 25 25 25 25	Domining Units Designing Units	92 15 6 224 35 11 12 12 12 12 12 12 12 12 12 12 12 12	35% 88% 33% 33% 33% 33% 33% 33% 33% 33% 33	50% 50% 50% 50% 50% 50% 50% 50% 50% 50%	47 4 4 7 4 7 4 7 5 9 8 4 4 9 1 1 2 5 2 8 1 1 1 1 1 2 1 1 1 1 1 2 1 1 1 1 1 1	68 12 4 188 24 1	11% 11% 11% 11% 11% 10% 10% 10% 10% 10%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 27 7 12 4 4 116 177 411 25 7 7 4 11 25 7 7 4 12 4 15 7 7 4 4 1 2 3 1 5 7 4 4 2 2 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1	19 7 7 59 12 4 15:22 2 8 2 4 8 8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	28 11 69 11
3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-B VC-B Other TC TC TC TC CC VC-C VC-C Other VC-B VC-C Other VC-B VC-B VC-B VC-B VC-B VC-B VC-B VC-B	Senior Acida Houseng-bissioned (\$125) Makillamin, Houseng-bissioned (\$125) Senior Acid Houseng-bissioned (\$125) Makillamin Houseng-bissioned (\$125) Senior Acid	455 254 33 619 82 59 215 225,6 1272 286,6 137 36,3 36,3 36,3 45,4 41,4 40,5 87 40,5 80,5 80,5 80,5 80,5 80,5 80,5 80,5 8	Demandry Units Design Units	92 16 6 6 224 36 16 17 17 17 17 17 17 17 17 17 17 17 17 17	35% 35% 35% 35% 35% 35% 35% 35% 35% 35%	27% 27% 27% 27% 27% 27% 27% 27% 27% 27%	47 12 14 17 15 15 17 15 16 17 17 18 18 18 19 19 19 19 19 19 19 19 19 19 19 19 19	68 12 4 166 24 4 2 851 166 167 167 167 167 167 167 167 167 16	11% 11% 12% 13% 10% 10% 10% 10% 10% 10% 10% 10% 10% 10	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 2 57 12 4 11 116 177 141 1177 141 125 74 5 5 7 74 2 122 122 122 59 110 18 7 99 13 5 15 16 18 7 99 13 5 16 18 7 99 13 5 18 18 7 99 13 5 18 18 18 7 99 13 5 18 18 18 18 18 18 18 18 18 18 18 18 18	19 7 66 12 4 152 23 6 2 24 4 1	28 11 80 11 81 81 81 81 81 81 81 81 81 81 81 81
3 3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC.B Other Other Other Other Other Other Other Other TG	Beninn Andel Houseng-bissimed (\$192) Makitamin, Houseng-bissimed (\$192) Samus Ander Houseng-bissimed (\$215) Samus Ander Houseng-bissimed (\$195) Samus Ander Houseng-bissimed (\$195) Samus Ander Houseng-bissimed (\$195) Samus Ander Houseng-Dissimed (\$195) Makitamin Houseng-Dissimed (\$195) Ma	455 254 33 4619 82 59 2155 2258.8 2155 1377 228.8 36.3 36.3 36.3 36.3 36.3 36.3 36.3 3	Demandry Units	92 16 6 6 224 34 17 2 12 2 12 2 12 12 12 15 15 15 15 15 15 15 15 15 15 15 15 15	35% 35% 35% 35% 35% 35% 35% 35% 35% 35%	27% 27% 27% 27% 27% 27% 27% 27% 27% 27%	4 2 2 3 3 4 4 5 5 5 6 6 7 7 4 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	68 12 4 4 186 24 187 187 187 187 187 187 187 187 187 187	11% 11% 11% 12% 13% 10% 10% 10% 10% 10% 10% 10% 10% 10% 10	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.6% 2.6% 2.6% 2.6% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5	4 23 6 2 2 57 12 4 11 116 177 117 117 117 117 117 117 117	19 7 66 12 4 152 23 8 64 152 25 25 17 17 17 16 16 16 17 17 17 17 17 17 17 17 17 17 18 18 18 18 18 18 18 18 18 18 18 18 18	28 11 89 18 8 219 219 219 24,5318 11007 857 357 117 864 11007 857 120 121 121 122 91 133 666 666 67 140 140 140 150 160 160 160 160 160 160 160 160 160 16
3 3 3 3 4 4 4 4	VC.B. Other	Benin Andel Houseng-bismined (\$125) Makitamin, Houseng-bismined (\$121) Samor Ande Houseng-bissished (\$121) Samor Ande Houseng-bissished (\$121) Samor Ande Houseng-bissished (\$121) Samor Ande Houseng-bissished (\$121) Samor Andel Houseng-Dissished (\$121) Samor Andel Houseng-Dissished (\$121) Samor Andel Houseng-Dissished (\$121) Samor Andel Houseng-Dissished Houseng-(\$10) Samor Andel Houseng-(\$120) Makitamin Houseng-(\$120) Samor Andel Houseng-(\$120) Samor Andel Houseng-(\$120) Makitamin Houseng-(\$120) Samor Andel Houseng-(\$120) Samor Andel Houseng-(\$120) Samor Andel Houseng-(\$120) Samor Andel Houseng-(\$120) Makitamin Houseng-(\$120) Samor Andel Houseng-(\$120) Makitamin Houseng-(\$120) Samor Andel Houseng-(\$120) Makitamin House	45 254 33 461 32 36 161 36 36 36 36 36 36 36 36 36 36 36 36 36	Demandry Units	92 16 6 6 224 18 18 212 212 212 18 18 18 18 18 18 18 18 18 18 18 18 18	35% 35% 35% 35% 35% 35% 35% 35% 35% 35%	27% 27% 27% 27% 27% 27% 27% 27% 27% 27%	# # # # # # # # # # # # # # # # # # #	68 12 4 4 186 24 187 187 187 187 187 187 187 187 187 187	50 00 00 00 00 00 00 00 00 00 00 00 00 0	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 2 57 12 4 1 16 177 18 6 9 9 17 18 18 18 18 18 18 18 18 18 18 18 18 18	19 7 7 56 12 4 152 23 6 5 2 481 5 1 1 2 1 2 1 5 1 2 1 2 1 1 2 1 1 2 1 2	28 11 89 219 12 12 12 12 12 12 12 12 12 12 12 12 12
3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-B Other	Beninn Ander Houseng-bissimole (1923) Makitamin, Havang-bissimole (1921) Samus Ander Houseng-bissimole (1921) Samus Ander Houseng-bissimole (1921) Samus Ander Houseng-bissimole (1921) Samus Ander Houseng-bissimole (1921) Samus Ander Houseng-Dissimole (1922) Samus Ander Houseng-Dissimole (1923)	455 254 43 33 46 19 254 45 215 215 215 215 215 216 216 216 216 216 216 216 216 216 216	Demandry Units	92 16 6 6 224 3.6 6 18 224 3.6 18 212 212 212 212 212 212 212 212 212	35% 35% 35% 35% 35% 35% 35% 35% 35% 35%	85% 85% 85% 85% 85% 85% 85% 85% 85% 85%	47 47 47 47 47 47 47 47 47 47 47 47 47 4	68 12 4 18 18 18 18 18 18 18 18 18 18 18 18 18	50 50 50 50 50 50 50 50 50 50 50 50 50 5	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	41 23 6 2 2 57 12 4 116 177 16 18 18 18 18 18 18 18 18 18 18 18 18 18	19 7 69 12 4 152 23 8 58 59 10 11 14 14 14 15 15 16 16 17 17 17 18 18 18 18 18 18 18 18 18 18 18 18 18	28 11 80 11 81 81 81 81 81 81 81 81 81 81 81 81
3 3 3 4 4 4 4 4 1 5 2 2 1 5 2	VC-B Other O	Senior Acida Sissama pilamined (\$125) Makilaminy Havang Senioral (\$125) Seniora Acid Hawang Dissisted (\$125) Seniora Child Hawang Dissisted (\$126) Senioral Child Landerd (\$126) Senioral Child Senioral (\$126) Senioral Child Seni	45) 254 43 33 43 45 254 254 254 442 253 442 254 254 254 255 277 277 277 277 277 277 277 277 277	Demining Units Design Units Des	92 18 6 6 224 38 18 2 2 2 2 3 5 2 2 2 4 18 18 2 2 2 2 4 18 18 2 2 2 2 4 18 18 2 2 2 2 4 18 18 2 2 2 2 2 4 18 18 2 2 2 2 2 4 18 18 2 2 2 2 3 2 3 3 3 3 3 3 3 3 3 3 3 3 3	32% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 33% 26% 36% 36% 36% 36% 36% 36% 36% 36% 36% 3	80% 10% 10% 10% 10% 10% 10% 10% 10% 10% 1	# # # # # # # # # # # # # # # # # # #	68 12 4 166 24 1 166 24 1 166 24 1 166 24 1 166 24 1 166 24 1 167 25 114 17 31 160 19 16 16 16 16 16 16 16 16 16 16 16 16 16	11% 11% 11% 11% 11% 11% 11% 10% 10% 10%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 97 12 4 1 16 177 18 5 5 12 18 18 18 18 18 18 18 18 18 18 18 18 18	19 7 66 12 4 162 23 6 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	28 11 89 11
3 3 3 4 4 4 4 4 1 5 2 2 2 2	VC-B VC-B Other Other Other Other Other Other Other Other TC TC TC TC TC CVC-C VC-C VC-C VC-C VC-	Benin Ander Skuszup-destinet (1923) Makitamin, Havang destinet (1923) Samus Ander Hauser (1924) Samus Ander Hauser (1925) Samus Ander Hauser (1926) Samus Ander Hauser (1926) Samus Ander Hauser (1926) Samus Ander Hauser (1926) Makitamin Hauser (1926) Samus Ander Hauser (1926) Samus Ander Hauser (1926) Samus Ander Hauser (1926) Samus Ander Hauser (1926) Makitamin Hauser (1926) Samus Ander Hauser (1926) Makitamin Hauser (1927) Makitamin Ha	45) 254 43 33 4 619 96 215 4 619 1 6	Demining think Demini	92 18 6 6 224 38 18 78 2 12 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	25% 25% 25% 25% 25% 25% 25% 25% 25% 25%	80% 80% 80% 80% 80% 80% 80% 80% 80% 80%	# # # # # # # # # # # # # # # # # # #	68 12 4 186 24 1 186 24 1 186 24 1 186 24 1 186 24 1 187 2 1	11% 11% 11% 12% 12% 12% 10% 10% 10% 10% 10% 10% 10% 10% 10% 10	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 7 7 2 8 1 1 1 1 1 1 1 2 1 2 1 2 1 2 1 2 1 2	19 7 69 12 4 62 22 23 2 8 8 8 930 59 9 17 17 49 6 22 24 17 17 18 18 18 18 18 18 18 18 18 18 18 18 18	28 11 89 219 12 12 12 12 12 12 12 12 12 12 12 12 12
3 3 3 4 4 4 4 4 4 1 6 2 2 1 6 2 2 1 6 2 2 2 1 8 2 1 8 2	VC-B VC-B Other Other Other Other Other Other TC TC TC TC CVC-C VC-C VC-C VC-C VC-C V	Senior Acida Houseng-destined (\$192) Makitaning Houseng-destined (\$192) Samura Acid Houseng-destined (\$215) Samura Acid Houseng-destined (\$195) Samura Acid Houseng-destined (\$195) Samura Acid Houseng-distined (\$105) Samura Acid Houseng-distined (455 254 33 261 89 89 89 89 128 2255 137 137 136 137 258.8 137 136 137 260 137 137 260 137 137 137 137 137 137 137 137 137 137	Demandry Units	92 18 6 6 224 23 2 22 22 22 22 24 2 18 9 6 14 4 26 10 19 9 14 2 10 19 9 14 2 10 19 9 14 2 10 19 9 14 2 10 19 10 10 10 10 10 10 10 10 10 10 10 10 10	25% 25% 25% 25% 25% 25% 25% 25% 25% 25%	80% 80% 80% 80% 80% 80% 80% 80% 80% 80%	34 3	68 12 4 18 18 18 18 18 18 18 18 18 18 18 18 18	11% 11% 11% 11% 11% 11% 11% 11% 11% 11%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 2 97 12 4 1 16 16 17 16 17 18 17 18 18 18 18 18 18 18 18 18 18 18 18 18	19 7 66 12 4 62 22 22 2 8 8 8 68 930 68 94 94 95 96 97 97 97 97 97 97 97 97 97 97 97 97 97	28 11 80 11 81 81 81 81 81 81 81 81 81 81 81 81
3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-B Other O	Beninn Andel Woutung-bill mind (\$125) Makillamin, Yanung-bill mind (\$125) Makillamin, Yanung-billamin (\$125) Makillamin,	45 254 23 24 25 25 25 25 25 25 25 25 25 25 25 25 25	Domaning Units Designing Units	92 18 6 6 244 35 8 4 7 8 18 18 18 18 18 18 18 18 18 18 18 18 1	25% 26% 25% 26% 26% 26% 26% 26% 26% 26% 26% 26% 26	85% 84% 84% 85% 85% 85% 85% 85% 85% 85% 85	# 24 # 2 # 2 # 2 # 2 # 2 # 2 # 2 # 2 # 2	68 12 4 6 12 14 16 6 12 18 18 18 18 18 18 18 18 18 18 18 18 18	25	25% 25% 25% 25% 25% 25% 25% 25% 25% 25%	4 23 6 2 7 2 9 2 9 2 9 2 9 2 9 2 9 2 9 2 9 2 9	19 7 7 50 12 4 4 1 1 1 2 2 1 2 4 1 4 1 1 2 2 1 2 4 1 4 1	28 11 80 11 81 81 81 81 81 81 81 81 81 81 81 81
3 3 3 4 4 4 4 4 4 162 2 1 1 2 2 3 3 3 3 3 4 4 5 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	VC-B VC-B VC-C VC-C VC-C VC-C VC-C VC-C	Senior Acida Sissama philamined (\$125) Makilaminy Humang Senioral (\$125) Seniora Acid Humang Glassined (\$125) Senioral Total acid Prisa Humang Senioral Acid Humang Senioral (\$126) Senioral College Senioral (\$126) Makilamin Hosonog (som-Bens) (\$126) Senioral College Senioral (\$126) Senioral College Senioral (\$126) Senioral Senioral Senioral (\$127) Senioral Senioral Senioral (\$127) Senioral Acid Senioral (\$127) Senioral Acid Senioral (\$127) Senioral Acid Senioral (\$127) Senioral Acid Senioral (\$127) Senioral Senioral Senioral (\$127)	455 254 33 619 619 625 627 628 619 628 629 628 629 628 629 628 629 629 629 629 629 629 629 629 629 629	Demining thin. Demining thin providing thin providing thin the providing that the providing the prov	92 18 6 6 224 34 18 18 18 18 18 18 18 18 18 18 18 18 18	25% 26% 25% 25% 26% 25% 26% 25% 26% 25% 25% 25% 25% 25% 25% 25% 25% 25% 25	85% 64% 65% 64% 65% 65% 65% 65% 65% 65% 65% 65% 65% 65	# 2 2 3 1 1 2 2 3 1 1 2 2 3 1 1 2 2 3 1 1 2 2 3 1 1 2 2 3 1 1 2 2 3 1 2	68 12 4 166 24 # # # 2.151 166 172 251 167 173 173 174 175 175 175 175 175 175 175 175 175 175	11% 11% 11% 10% 10% 10% 10% 10% 10% 10%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 2 57 2 4 1 1 1 1 1 1 1 2 1 2 1 2 1 2 1 2 1 2	19 7 69 12 4 162 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	28 11 89 11
3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VC-B Other O	Benin Andel Houseng-bissined (\$125) Makitanin, Houseng-bissined (\$125) Samus Andel Houseng-Dissined Houseng-(\$120) Samus Andel Houseng-(\$120) Makitanin Houseng-(\$120) Samus Andel Houseng-(\$120	45 254 23 3 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2	Demahring United Demahr	92 18 6 6 224 23 24 21 21 21 21 21 21 21 21 21 21 21 21 21	25% 25% 25% 25% 25% 25% 25% 25% 25% 25%	65% 65% 65% 65% 65% 65% 65% 65% 65% 65%	# # # # # # # # # # # # # # # # # # #	68 12 4 166 24 4 17 18 18 18 18 18 18 18 18 18 18 18 18 18	11% 11% 13% 13% 10% 10% 10% 10% 10% 10% 10% 10% 10% 10	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 7 7 124 115 127 124 125 127 127 124 115 127 127 127 128 128 128 128 128 128 128 128 128 128	19 7 7 50 12 4 8 11 22 2 8 8 12 12 2 8 12 12 12 12 12 12 12 12 12 12 12 12 12	28 18 28 28 28 28 28 28 28 28 28 28 28 28 28
3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	VG-B VG-B Other	Beninn Andel Houseng-billion (ESP2) Makillamin, Houseng-billion (ESP2) Makillamin, Houseng-billion (ESP2) Service And Houseng-billion (ESP2) Service And Houseng-billion (ESP2) Service Andel Houseng-Dissipated (ESP2) Service Andel Houseng-Dissipat	45 45 254 33 15 254 254 254 255	Domining Units Designing Units	92 19 6 6 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	20% 28% 28% 28% 28% 28% 28% 28% 28% 28% 28	50% 50% 50% 50% 50% 50% 50% 50% 50% 50%	# # # # # # # # # # # # # # # # # # #	68 12 4 166 4 2 4 1 166 4 2 4 1 166 4 2 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	11% 11% 11% 11% 11% 11% 11% 11% 11% 11%	2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5% 2.5%	4 23 6 2 7 2 7 2 8 1 1 1 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2	19 7 7 60 21 12 4 162 2 2 1 2 4 16 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2 1 2	28 11 89 219 315 32 4,533 31 31 31 31 32 32 32 32 32 32 32 32 32 32 32 32 32

	NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Olympia Hills		Organization:	Hales Engineering						
Project Location:	Salt Lake County		Performed By:	Josh Gibbons						
Scenario Description:	Town Center Area		Date:	12/6/2019						
Analysis Year:	2042		Checked By:							
Analysis Period:	AM Street Peak Hour		Date:							

Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs1	Quantity	Units	Total	Entering	Exiting
Office	710	1,272	1,000 sq ft	1,224	1,053	171
Retail	820	258.8	1,000 sq ft	244	151	93
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	914	dwelling units	430	112	318
Hotel				0		
All Other Land Uses ²				0		
				1,898	1,316	582

Table 2-A: Mode Split and Vehicle Occupancy Estimates										
Land Use		Entering Tri	ps		Exiting Trips					
Land Ose	Veh. Occ.4	% Transit	% Non-Motorized		Veh. Occ.4	% Transit	% Non-Motorized			
Office	1.06	2.5%	0%	ĺ	1.06	2.5%	0%			
Retail	1.17	2.5%	0%	1	1.17	2.5%	0%			
Restaurant										
Cinema/Entertainment				- 1						
Residential	1.13	2.5%	0%		1.13	2.5%	0%			
Hotel										
All Other Land Uses ²										

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)										
Origin (From)		Destination (To)								
Oligin (Florii)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office										
Retail										
Restaurant										
Cinema/Entertainment										
Residential						TALL TO THE				
Hotel	1000									

Table 4-A: Internal Person-Trip Origin-Destination Matrix*										
Origin (From)		Destination (To)								
Oligili (Fiolil)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		51	0	0	0	0				
Retail	32		0	0	3	0				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	7	4	0	0		0				
Hotel	0	0	0	0	0					

Table 5-A: Computations Summary									
	Total	Entering	Exiting						
All Person-Trips	2,069	1,420	649						
Internal Capture Percentage	9%	7%	15%						
External Vehicle-Trips ⁵	1,682	1,200	482						
External Transit-Trips ⁶	47	33	14						
External Non-Motorized Trips ⁶	0	0	0						

Table 6-A: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	3%	28%						
Retail	31%	32%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	2%	3%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool										
Project Name:	Olympia Hills	Organization:	Hales Engineering								
Project Location:	Salt Lake County		Performed By:	Josh Gibbons							
Scenario Description:	Town Center Area		Date:	12/6/2019							
Analysis Year:	2042		Checked By:								
Analysis Period:	PM Street Peak Hour		Date:								

Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs1	Quantity	Units	Total	Entering	Exiting
Office	710	1,272	1,000 sq ft	1,276	204	1,072
Retail	820	258.8	1,000 sq ft	988	474	514
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	914	dwelling units	542	337	205
Hotel				0		
All Other Land Uses ²				0		
				2,806	1,015	1,791

Table 2-P: Mode Split and Vehicle Occupancy Estimates										
4 411		Entering Tri	ps		Exiting Trips					
Land Use	Veh. Occ.4	% Transit	% Non-Motorized	Veh. Occ.⁴	% Transit	% Non-Motorized				
Office	1.11	2.5%	0%	1.11	2.5%	0%				
Retail	1.21	2.5%	0%	1.21	2.5%	0%				
Restaurant										
Cinema/Entertainment										
Residential	1.15	2.5%	0%	1.15	2.5%	0%				
Hotel										
All Other Land Uses ²										

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)										
Origin (Fram)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		2000			2000					
Retail					2000					
Restaurant										
Cinema/Entertainment						100				
Residential		2000				Œ				
Hotel										

Table 4-P: Internal Person-Trip Origin-Destination Matrix*										
Origin (Frank)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		15	0	0	15	0				
Retail	12		0	0	102	0				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	9	18	0	0		0				
Hotel	0	0	0	0	0					

Table 5-P: Computations Summary									
	Total	Entering	Exiting						
All Person-Trips	3,236	1,188	2,048						
Internal Capture Percentage	11%	14%	8%						
External Vehicle-Trips ⁵	2,451	846	1,605						
External Transit-Trips ⁶	73	26	47						
External Non-Motorized Trips ⁶	0	0	0						

Table 6-P: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	9%	3%						
Retail	6%	18%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	30%	11%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Ołympia Hills		Organization:	Hales Engineering					
Project Location:	Salt Lake County		Performed By:	Josh Gibbons					
Scenario Description:	Village Center A Area		Date:	12/6/2019					
Analysis Year:	2042		Checked By:						
Analysis Period:	AM Street Peak Hour		Date:						

Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	90.1	1,000 sq ft	112	96	16
Retail	820	45.4	1,000 sq ft	44	27	17
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	630	dwelling units	445	110	335
Hotel				0		
All Other Land Uses ²				0		
				601	233	368

Table 2-A: Mode Split and Vehicle Occupancy Estimates										
Land Use		Entering Tri	ps	П		Exiting Trips				
Land Use	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized			
Office	1.06	2.5%	0%		1.06	2.5%	0%			
Retail	1.17	2.5%	0%		1.17	2.5%	0%			
Restaurant				li						
Cinema/Entertainment										
Residential	1.13	2.5%	0%		1.13	2.5%	0%			
Hotel										
All Other Land Uses ²										

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)										
Origin (From)				Destination (To)						
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office										
Retail										
Restaurant										
Cinema/Entertainment										
Residential										
Hotel										

Table 4-A: Internal Person-Trip Origin-Destination Matrix*										
Origin (From)		Destination (To)								
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		5	0	0	0	0				
Retail	4		0	0	2	0				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	3	4	0	0		0				
Hotel	0	0	0	0	0					

Table 5-A: Computations Summary									
	Total	Entering	Exiting						
All Person-Trips	674	258	416						
Internal Capture Percentage	5%	7%	4%						
External Vehicle-Trips ⁵	556	212	344						
External Transit-Trips ⁶	15	6	9						
External Non-Motorized Trips ⁶	0	0	0						

Table 6-A: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	7%	29%						
Retail	28%	30%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	2%	2%						
Hotel	N/A	N/A						

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool									
Project Name:	Project Name: Olympia Hills Organization: Hales En									
Project Location:	ion: Salt Lake County		Performed By:	Josh Gibbons						
Scenario Description:	Village Center A Area		Date:	10/22/2019						
Analysis Year:	2042		Checked By:	Scott Johnson						
Analysis Period:	PM Street Peak Hour		Date:	10/22/2019						

	Table 1-	P: Base Vehic	le-Trip Generation E	stimates (Single-Use Sit	e Estimate)	
Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs1	Quantity	Units	Total	Entering	Exiting
Office	710	90.1	1,000 sq ft	104	17	87
Retail	820	45.4	1,000 sq ft	174	84	90
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	630	dwelling units	384	238	146
Hotel				0		
All Other Land Uses ²				0		
				662	339	323

Table 2-P: Mode Split and Vehicle Occupancy Estimates										
Land Use		Entering Tri	ps	П		Exiting Trips				
	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized			
Office	1.11	2.5%	0%		1.11	2.5%	0%			
Retail	1.21	2.5%	0%		1.21	2.5%	0%			
Restaurant										
Cinema/Entertainment										
Residential	1.15	2.5%	0%		1.15	2.5%	0%			
Hotel										
All Other Land Uses ²										

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)										
Origin (From)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		1750			1750					
Retail					1750					
Restaurant										
Cinema/Entertainment										
Residential		1750								
Hotel										

Table 4-P: Internal Person-Trip Origin-Destination Matrix*									
Origin (From)		Destination (To)							
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		4	0	0	1	0			
Retail	2		0	0	20	0			
Restaurant	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0			
Residential	7	5	0	0		0			
Hotel	0	0	0	0	0				

Table 5-P: Computations Summary							
	Total	Entering	Exiting				
All Person-Trips	769	395	374				
Internal Capture Percentage	10%	10%	10%				
External Vehicle-Trips ⁵	582	299	283				
External Transit-Trips ⁶	16	8	8				
External Non-Motorized Trips ⁶	0	0	0				

Table 6-P: Internal Trip Capture Percentages by Land Use							
Land Use	Entering Trips	Exiting Trips					
Office	47%	5%					
Retail	9%	20%					
Restaurant	N/A	N/A					
Cinema/Entertainment	N/A	N/A					
Residential	8%	7%					
Hotel	N/A	N/A					

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip (Cap	ture Estimation Tool	
Project Name:	Olympia Hills		Organization:	Hales Engineering
Project Location:	Salt Lake County		Performed By:	Josh Gibbons
Scenario Description:	Village Center B Area		Date:	12/6/2019
Analysis Year:	2042		Checked By:	
Analysis Period:	AM Street Peak Hour		Date:	

Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
	ITE LUCs1	Quantity	Units	Total	Entering	Exiting
Office				0		
Retail	820	40.5	1,000 sq ft	40	25	15
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	972	dwelling units	454	118	336
Hotel				0		
All Other Land Uses ²				0		
				494	143	351

	- 0	Table 2-A:	Mode Split and Veh	icle	Occupancy Estimates			
Land Use		Entering Tri	ps		Exiting Trips			
	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.4	% Transit	% Non-Motorized	
Office								
Retail	1.17	2.5%	0%		1.17	2.5%	0%	
Restaurant								
Cinema/Entertainment								
Residential	1.13	2.5%	0%		1.13	2.5%	0%	
Hotel								
All Other Land Uses ²								

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)								
Origin (From)		Destination (To)						
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel		
Office								
Retail								
Restaurant								
Cinema/Entertainment								
Residential								
Hotel			0.2					

Table 4-A: Internal Person-Trip Origin-Destination Matrix*									
Origin (From)		Destination (To)							
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		0	0	0	0	0			
Retail	0	4-1-1-1	0	0	3	0			
Restaurant	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0			
Residential	0	4	0	0		0			
Hotel	0	0	0	0	0				

Table 5-A: Computations Summary								
	Total	Entering	Exiting					
All Person-Trips	560	162	398					
Internal Capture Percentage	3%	4%	2%					
External Vehicle-Trips ⁵	471	133	338					
External Transit-Trips ⁶	13	4	9					
External Non-Motorized Trips ⁶	0	0	0					

Table 6-A: Internal Trip Capture Percentages by Land Use							
Land Use	Exiting Trips						
Office	N/A	N/A					
Retail	14%	17%					
Restaurant	N/A	N/A					
Cinema/Entertainment	N/A	N/A					
Residential	2%	1%					
Hotel	N/A	N/A					

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*),

Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number-

	NCHRP 684 Internal Trip Capture Estimation Tool						
Project Name:	Olympia Hills		Organization:	Hales Engineering			
Project Location:	Salt Lake County		Performed By:	Josh Gibbons			
Scenario Description:	Village Center B Area	1	Date:	12/6/2019			
Analysis Year:	2042	1	Checked By:				
Analysis Period:	PM Street Peak Hour		Date:				

Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
	ITE LUCs1	Quantity	Units	Total	Entering	Exiting
Office				0		
Retail	820	40.5	1,000 sq ft	156	75	81
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	972	dwelling units	574	355	219
Hotel				0		
All Other Land Uses ²				0		
				730	430	300

		Table 2-P:	Mode Split and Veh	ick	e Occupancy Estimates			
Land Use		Entering Tri	ps		Exiting Trips			
Land Ose	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized	
Office								
Retail	1.21	2.5%	0%		1.21	2.5%	0%	
Restaurant								
Cinema/Entertainment								
Residential	1.15	2.5%	0%		1.15	2.5%	0%	
Hotel								
All Other Land Uses2								

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)							
Origin (From)				Destination (To)			
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel	
Office							
Retail					1500		
Restaurant							
Cinema/Entertainment			To the same of			- 1	
Residential		1500					
Hotel							

Table 4-P: Internal Person-Trip Origin-Destination Matrix*								
Origin (Fram)			,	Destination (To)				
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel		
Office		0	0	0	0	0		
Retail	0		0	0	20	0		
Restaurant	0	0		0	0	0		
Cinema/Entertainment	0	0	0		0	0		
Residential	0	6	0	0		0		
Hotel	0	0	0	0	0			

Table 5-P: Computations Summary							
	Total	Entering	Exiting				
All Person-Trips	849	499	350				
Internal Capture Percentage	6%	5%	7%				
External Vehicle-Trips ⁵	670	398	272				
External Transit-Trips ⁶	20	12	8				
External Non-Motorized Trips ⁶	0	0	0				

Table 6-P: Internal Trip Capture Percentages by Land Use						
Land Use	Entering Trips	Exiting Trips				
Office	N/A	N/A				
Retail	7%	20%				
Restaurant	N/A	N/A				
Cinema/Entertainment	N/A	N/A				
Residential	5%	2%				
Hotel	N/A	N/A				

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

Enter trips assuming no transit or non-motorized trips (as assumed in ITE Trip Generation Manual).

Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip C	ap [°]	ture Estimation Tool	
Project Name:	Olympia Hills		Organization:	Hales Engineering
Project Location:	Salt Lake County		Performed By:	Josh Gibbons
Scenario Description:	Village Center C Area		Date:	12/6/2019
Analysis Year:	2042		Checked By:	,
Analysis Perlod:	AM Street Peak Hour		Date:	

Land Use	Developme	nt Data (For In	formation Only)		Estimated Vehicle-Trips ³	
Land Ose	ITE LUCs1	Quantity	Units	Total	Entering	Exiting
Office	710	31.9	1,000 sq ft	58	50	8
Retail	820	36.3	1,000 sq ft	36	22	14
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	576	dwelling units	276	72	204
Hotel				0		
All Other Land Uses ²				0		
				370	144	226

		Table 2-A:	Mode Split and Veh	nicle	Occupancy Estimates			
1 . 111		Entering Tri	ps			Exiting Trips		
Land Use	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized	
Office	1.06	2.5%	0%	Ī	1.06	2.5%	0%	
Retail	1.17	2.5%	0%		1.17	2.5%	0%	
Restaurant								
Cinema/Entertainment								
Residential	1.13	2.5%	0%		1.13	2.5%	0%	
Hotel								
All Other Land Uses ²								

Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)								
Origin (France)				Destination (To)				
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel		
Office								
Retail								
Restaurant								
Cinema/Entertainment								
Residential								
Hotel		× ,						

		Table 4-A: I	nternal Person-Tri	p Origin-Destination Matrix*						
Origin (From)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office		2	0	0	0	0				
Retail	2		0	0	2	0				
Restaurant	0	0		0	0	0				
Cinema/Entertainment	0	0	0		0	0				
Residential	2	2	0	0		0				
Hotel	0	0	0	0	0					

Table 5-A: Computations Summary								
	Total	Entering	Exiting					
All Person-Trips	415	160	255					
Internal Capture Percentage	5%	6%	4%					
External Vehicle-Trips ⁵	343	131	212					
External Transit-Trips ⁶	10	4	6					
External Non-Motorized Trips ⁶	0	0	0					

Table 6-A: Internal Trip Capture Percentages by Land Use						
Land Use	Entering Trips	Exiting Trips				
Office	8%	25%				
Retail	15%	25%				
Restaurant	N/A	N/A				
Cinema/Entertainment	N/A	N/A				
Residential	2%	2%				
Hotel	N/A	N/A				

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

	NCHRP 684 Internal Trip Capture Estimation Tool							
Project Name:	Olympia Hills	Hales Engineering						
Project Location:	Salt Lake County		Performed By:	Josh Gibbons				
Scenario Description:	Village Center C Area		Date:	12/6/2019				
Analysis Year:	2042		Checked By:					
Analysis Period:	PM Street Peak Hour		Date:					

Land Use	Developme	nt Data (For In	formation Only)	Estimated Vehicle-Trips ³		
	ITE LUCs ¹	Quantity	Units	Total	Entering	Exiting
Office	710	31.9	1,000 sq ft	40	6	34
Retail	820	36.3	1,000 sq ft	140	67	73
Restaurant				0		
Cinema/Entertainment				0		
Residential	,220,221,251,	576	dwelling units	351	218	133
Hotel				0		
All Other Land Uses ²				0		
				531	291	240

		Table 2-P:	Mode Split and Vehi	icle O	ccupancy Estimates		
Land Use		Entering Tr	ips		Exiting Trips		
	Veh. Occ.⁴	% Transit	% Non-Motorized		Veh. Occ.⁴	% Transit	% Non-Motorized
Office	1.11	2.5%	0%		1.11	2.5%	0%
Retail	1.21	2.5%	0%		1.21	2.5%	0%
Restaurant							
Cinema/Entertainment							
Residential	1.15	2.5%	0%		1.15	2.5%	0%
Hotel							
All Other Land Uses ²							

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)									
Origin (From)		Destination (To)							
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		1000			1000				
Retail					1000				
Restaurant									
Cinema/Entertainment									
Residential		1000		Garage II					
Hotel									

		Table 4-P: I	nternal Person-Tri	p Origin-Destination Matrix*		
Origin (From)				Destination (To)		
	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel
Office		5	0	0	1	0
Retail	2		0	0	21	0
Restaurant	0	0		0	0	0
Cinema/Entertainment	0	0	0		0	0
Residential	4	6	0	0		0
Hotel	0	0	0	0	0	

Table 5-P: Computations Summary						
	Total	Entering	Exiting			
All Person-Trips	618	339	279			
Internal Capture Percentage	13%	12%	14%			
External Vehicle-Trips ⁵	452	251	201			
External Transit-Trips ⁶	15	8	7			
External Non-Motorized Trips ⁶	0	0	0			

Table 6-P: Internal Trip Capture Percentages by Land Use				
Land Use	Entering Trips	Exiting Trips		
Office	86%	16%		
Retail	14%	26%		
Restaurant	N/A	N/A		
Cinema/Entertainment	N/A	N/A		
Residential	9%	7%		
Hotel	N/A	N/A		

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE Trip Generation Manual).

Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Exhibit "E"

Regional Compatibility Plan and Guidelines

- 1.1. Master Developer and the County have, through the zoning of the Planned Community and the adoption of this MDA have intended to respect existing communities and neighborhoods. Through the subsequent adoptions of CSPs, Project Plans, Site Plans, and subdivision plats, the Parties shall further respect existing communities and neighborhoods. This shall be achieved by acknowledging important components of these areas in the planning and design of Planned Community (e.g., their history, established direction, significant places/features and views, and relationship to other communities).
- 1.2. Master Developer in future CSPs, Project Plans, Site Plans, and subdivision plats shall understand existing conditions in neighboring cities and developments and be a part of collaborative solutions for features that commonly link one community/neighborhood with another, such as transportation, parks, trails, utilities, etc.
- 1.3. Master Developer and the County shall work together to make all future Project Plans/Subdivision Plats/Site Plans compatible with the General Plan as modified by the P-C Zoning, this MDA and any future CSPs.
- 1.4. Community Structure Plan(s) and Project Plan(s)/Subdivision Plats/Site Plans shall be consistent with the General Plan, and WFRC's current Regional Transportation Plan (RTP) and Transportation Improvement Program (TIP).

Exhibit "F"

Parking Authority and Parking Policies

- 1.1 A Parking Authority Management Plan shall be required as part of a CSP for a Town Center, Commercial Center, and Village Center, but not for a Neighborhood (as those terms are used in Exhibit C of this MDA).
- 1.2 The CSP shall provide provisions for the governance of the Parking Authority. The Parking Authority shall be a public private partnership. The Developer representation of the Parking Authority shall manage off street parking and the County representation shall manage on street parking.
- 1.3 Subject to any modification in a future CSP, the following parking policies apply to the development and shall be part of the Parking Authority Management Plan
 - 1.3.1 On-street parking, which generally reduces traffic speeds and provides easy access for quick-stop shopping, shall be provided according to MDA or CSP Design Standards in all centers and destinations.
 - 1.3.2 Although surface parking lots are permitted in Town and Village Centers, structured parking and subterranean or semi-depressed garages are encouraged wherever economically practicable. Community Structure Plan(s) shall implement Design Standards relating to surface parking lots, by, among other design elements, determining when some or all buildings should front the street with doors facing the street and parking located behind or between buildings and occupying only a limited portion of the street frontage.
 - 1.3.3 Shared parking strategies shall be used when there are adjoining land uses with different periods of peak activity in order to accommodate parking demand.
 - 1.3.4 The location and design of off-street parking facilities in residential districts shall mitigate visual intrusion into the public right-of-way and community spaces. Parking for multi-family, civic, and commercial buildings shall generally be located in structures, underground facilities, or in locations obscured from street view by buildings or landscaping. Local streets may include on-street parking to accommodate visitors and serve as a buffer between street and sidewalk.
 - 1.3.5 The design of surfaced and structured parking shall be according to MDA or CSP Design Standards and shall be well-landscaped, incorporating shade trees, shrubs, perennials and other plants and treatments to reduce the negative impacts of the surface lots and structured parking areas.
 - 1.3.6 The design of surfaced and structured parking shall accommodate and prioritize alternative transportation modalities such as ride-sharing, transportation network company (TNC) drop-off and pick-up zones, EV-charging and mass transit.
 - 1.3.7 CSP(s) shall implement Design Standards intended to accomplish the following: the location and design of off-street parking facilities in

residential districts shall minimize visual intrusion into the public right-of-way and community spaces; parking for multi-family, civic, and commercial buildings shall be located in structures, underground facilities, or in locations obscured from street view by buildings or landscaping; and local streets may include on-street parking to accommodate visitors and serve as a buffer between street and sidewalk.

Exhibit "G"

County's Vested Laws

Exhibit "H"

Affordable/Workforce Housing Plan

- 1.1 Olympia Hills shall use an inclusionary approach that allows for a mixture of housing types and prices distributed throughout the communities of Olympia Hills, as well as near employment centers, recognizing that housing affordability is integral to the long-term success of Olympia Hills and the region. Olympia Hills is committed to helping ease the affordable housing problem including by using the economic and planning advantages of being a master-planned community.
- 1.2 Both attached and detached Accessory Dwelling Units (ADUs) are allowed; being secondary or ancillary units, ADUs must be compatible in architectural style to the single-family home they accompany. ADUs will be subject to future ADU ordinances, which shall include design standards for ADUs. Building typologies for ADUs shall be determined at the CSP.
- 1.3 Affordable housing units in various types of housing stock shall be provided through incentive programs and/or partnerships with a range of entities, including home builders, developers, non-profit organizations, and public agencies (such as the Olene Walker Fund and tax credits).
- 1.4 Developer shall encourage major employers locating within Olympia Hills to develop employer-assisted housing programs for lower income employees.
- 1.5 A minimum of 10% of the total number of approved housing units shall be Affordable Units reserved for households earning between 0% and 80% of the Area Median Income (AMI) as determined by the annual updated HUD level incomes. The average income limit for all Affordable Units for rent (per phase) shall not exceed 60% AMI. Notwithstanding the previous restrictions, any RDU that meets the requirements of IRC Section 42 and is eligible for low income housing credits (LIHTCs) shall automatically qualify as an Affordable Unit. Each Affordable Unit for rent shall be subject to the same income restrictions for a period of 30 years, or for a term determined by the Utah State tax credit administrative agency or other applicable low-income housing program sponsor, whichever is longer.
 - 1.5.1 Affordable Units shall be developed roughly proportionate with market units and interspersed at each phase. The Planned Community shall have a mix of Affordable Units for rent (minimum of 30%) and for sale (minimum of 30%). Affordable Units for sale need only comply with the 0%-80% AMI requirement in section 1.5 and no average AMI is required. Affordable Units for sale are not subject to a deed restriction, but initial purchasers' incomes must comply with the 0%-80% AMI requirements. Affordable Units for sale shall be individually platted and may include condominiums, townhomes, single family homes, or other types of for-sale units.
- 1.6 5% of the total number of approved housing units shall be reserved for Workforce Units for households earning between 80% and 120% of the Area Median Income (AMI) as determined by HUD. Workforce Units can be for rent or for sale. Workforce Units shall be developed roughly proportionate with market units and interspersed at each phase. Developer is encouraged to work with employers and builders to facilitate community-based housing within Olympia Hills.

1.7 Developer shall implement strategies as part of CSPs or Project Plans to encourage and/or require the levels of Affordable Housing specified herein. The results of these strategies, as well as methods of ensuring that Affordable Housing remains affordable while recognizing the desires and needs of homeowners to build equity, will produce the results outlined in this Exhibit. Developer shall submit an Affordable Housing report and proposed plan for how the requirements of this Exhibit will be accomplished, and shall submit that report and plan with a Community Structure Plan.